



GEOTECHNICAL SUPPLEMENT NO. 1: STORMWATER INFILTRATION

Central Kitsap High School and Middle School Redevelopment
10130 Frontier Place NW and 3700 NW Anderson Hill Road
Silverdale, Washington

Prepared for:

Central Kitsap School District #401

Jean-Wright Administration Center
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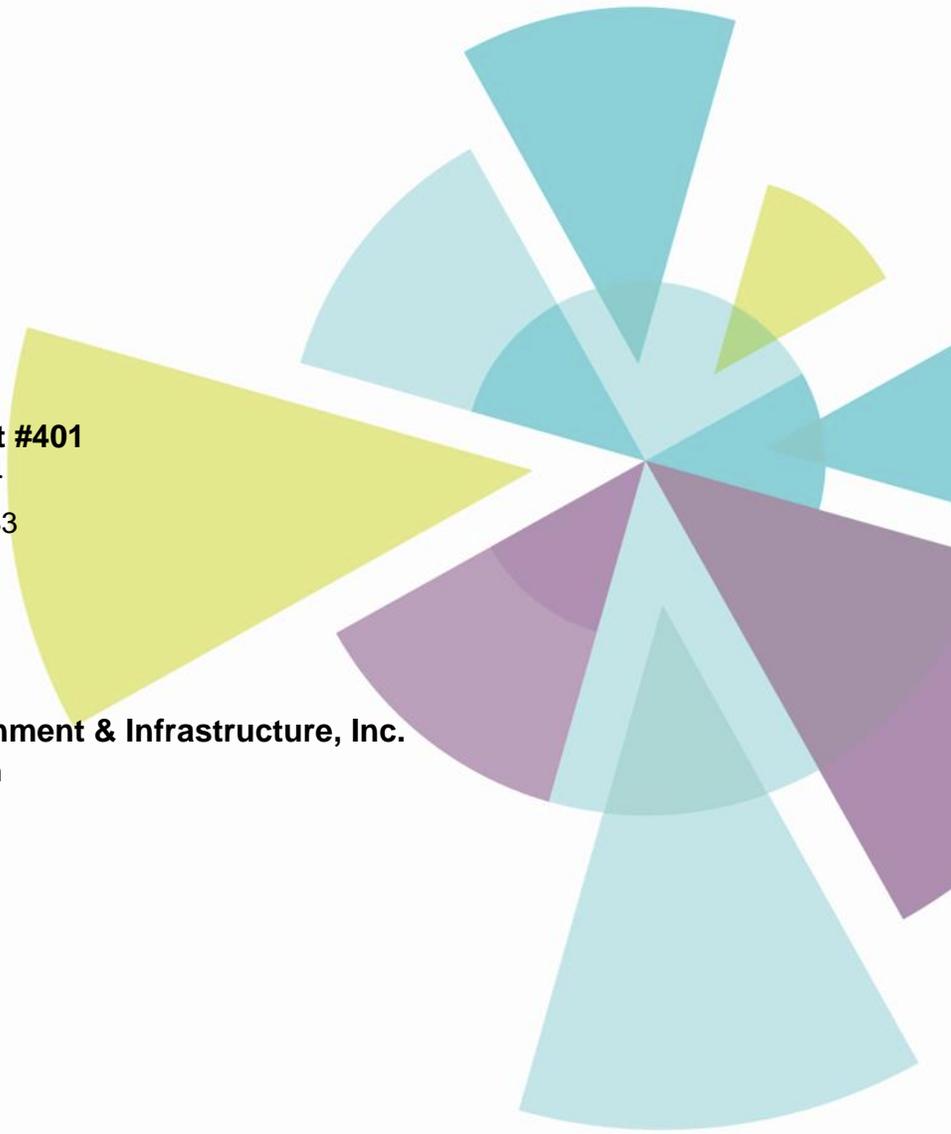
Prepared by:

Amec Foster Wheeler Environment & Infrastructure, Inc.

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March 10, 2017

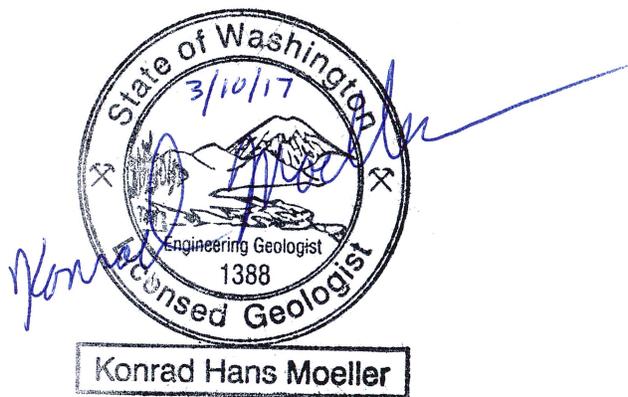
Project No. 6-917-18096-0



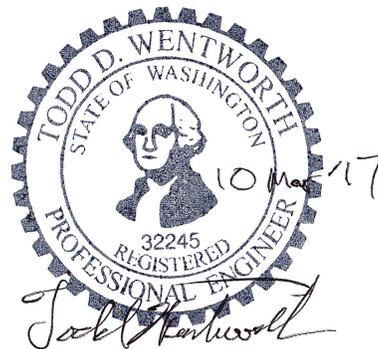
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EXECUTIVE SUMMARY

Amec Foster Wheeler Environment & Infrastructure, Inc., is please to submit this Geotechnical Supplemental No.1: Stormwater Infiltration report to Central Kitsap School District (CKSD) describing the results of groundwater measurements and infiltration testing for the Central Kitsap High School and Middle School redevelopment project. We previously completed a geotechnical engineering evaluation for the redevelopment titled *Preliminary Geotechnical Engineering Report, Central Kitsap High School and Middle School Campus Redevelopment*, dated October 18, 2016. The purpose of this supplement is to estimate infiltration rates for use in the design of infiltration facilities proposed for the redevelopment project. This executive summary is presented for introductory purposes and should be used only in conjunction with the full text of this report.

Project Description: The redevelopment plans provided call for construction of three infiltration ponds and one wet pond on the west side of the site near NW Anderson Hill Road. The ponds will be located at the existing Alternative High School Building (north infiltration pond), Science Building (central infiltration pond), and high school staff parking lot (south infiltration pond).

Summary of Infiltration Pond Results and Recommendations: Based on the soil and groundwater conditions encountered and results of pilot infiltration tests (PITs) completed at each location, we arrived at the following conclusions:

- North infiltration pond – Results of PITs, laboratory testing, and engineering analysis indicate infiltration is feasible using a long-term design infiltration rate of 1 inch per hour. The vertical groundwater separation of the pond base and highest groundwater reading from observation well OW-3 at the pond is 11 feet. Laboratory testing results for samples of soil underlying the pond showed organic content of 0.5 and 1.1 percent and cation exchange capacity (CEC) of 1.9 and 6.7 milliequivalents per 100 grams (meq/100 g). Fill was encountered to depths of 8.5 feet below the ground surface near the pond southwest corner. Because this pond is situated next to the top of a steep slope, we have recommended constructing a fill embankment to decrease the risk of seepage on the slope below the pond.
- Central infiltration pond – Results of PITs, laboratory testing, and engineering analysis indicate slow infiltration at this location, resulting in a long-term design infiltration rate of 0.16 inch per hour. The vertical groundwater separation of the pond base and highest groundwater reading from OW-1 just west of the pond is 5.5 feet. Laboratory testing results for a sample of the soil underlying the pond showed an organic content of 0.7 percent and a CEC of 1.2 meq/100g. Fill was encountered to 14 feet below the ground surface near the pond northwest corner. If fill is present at the pond subgrade, the fill should be overexcavated until the Advance Outwash soils are exposed.

- South infiltration pond – Results of PITs, laboratory testing, and engineering analysis indicate infiltration is feasible using a long-term design infiltration rate of 0.84 inch per hour. The vertical groundwater separation of the pond base and highest groundwater reading from observation well OW-2 at the pond is 4.5 feet. Laboratory testing of a sample of the soil underlying the pond showed an organic content of 0.7 percent and CEC of 2.6 meq/100g.

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GEOTECHNICAL SUPPLEMENT NO. 1: STORMWATER INFILTRATION

Central Kitsap High School and Middle School Redevelopment Silverdale, Washington

1.0 PROJECT DESCRIPTION

Amec Foster Wheeler Environment & Infrastructure, Inc. (Amec Foster Wheeler), is pleased to submit this supplemental report describing the results of groundwater measurements and infiltration testing for the Central Kitsap High School and Middle School Redevelopment project located in Silverdale, Washington. The purpose of our testing was to recommend an appropriate infiltration rate to be used in design of infiltration facilities planned for campus redevelopment, as shown on the enclosed Site Location Map (Figure 1). We previously completed a preliminary geotechnical engineering evaluation for the development (Amec Foster Wheeler, 2016). After completing our preliminary work, Central Kitsap School District (CKSD) requested additional exploration, infiltration testing, and analysis to estimate long-term design infiltration rates for proposed stormwater infiltration facilities. Additional subsurface explorations, infiltration testing, and analysis for long-term infiltration rates were completed in general accordance with requirements outlined in the Kitsap County Stormwater Manual, 2010 edition (Kitsap County, 2010), as requested by the project civil engineering firm, AHBL.

The current infiltration facility design is composed of three infiltration ponds and one wet pond. The proposed infiltration ponds are located at the existing Alternative High School Building (north infiltration pond), the existing Science Building (central infiltration pond), and the current high school staff parking lot (south infiltration pond) (Figure 2). The civil plans we received on March 2, 2017 call for the following design infiltration depths and surface areas:

- North infiltration pond: 7 feet below final grade (elevation 119 feet), with a base surface area of 25,379 square feet (sq.ft.);
- Central infiltration pond: 8 feet below final grade (elevation 113 feet), with a pond base area of 6,021 sq.ft; and
- South infiltration pond: varies from 7 to 9 feet below final grade (elevation 102 to 104 feet), with a base surface area of 13,348 sq.ft.

This study evaluated the subsurface conditions at all three infiltration pond locations. Our study did not include any subsurface evaluation or engineering for the planned wet pond.

As outlined in our proposals dated October 20, 2016, and February 14, 2017, our scope of work for this study included drilling soil borings, well installation, excavation of test pits, pilot infiltration tests (PITs), laboratory testing, data interpretation, and report preparation. To evaluate the subsurface soil and infiltration conditions at each infiltration pond location, we conducted four large-ring in situ PITs, designated IT-1 through IT-4. Two PITs were completed at the north infiltration pond, and one PIT each was completed at the south and central infiltration ponds.

The field work was performed on December 7, 2016, December 21 through 23, 2016, and February 20, 2017. Prior to, during, and after completing our field work, AHBL redesigned the infiltration facility multiple times. The exploration locations and testing were based on the infiltration pond locations, depths, and sizes provided by AHBL prior to our field work, and some of the locations were adjusted as the design progressed. The design infiltration facilities based on civil drawings received on March 2, 2017 are illustrated in the attached site and exploration plan (Figure 2).

2.0 SUBSURFACE SOIL CONDITIONS

The published geologic map for the site vicinity (Polenz et al., 2013) shows the infiltration facility area to be in or near the demarcation line separating Possession Advance Outwash (Qgap) on the western portion of the site and Pleistocene Vashon Lodgment Till (Qgt) on the east side of the site. Our explorations encountered both Possession Advance Outwash and Vashon Lodgment Till soils. For the purpose of this report, the Vashon Lodgment Till will be referred to as *Glacial Till* or *Sandy Glacial Till* and the Possession Advance Outwash will be referred to as *Advance Outwash*. All Advance Outwash encountered in this study contained thin layers or lenses of very dense silty sand or hard silt at variable depths.

In general, our explorations encountered fill over Glacial Till at the north and central ponds, and fill over Advance Outwash at the south pond. The approximate locations of our exploration borings, test pits, and infiltration pits are shown on Figure 2. This section summarizes the subsurface soil conditions encountered at each infiltration pond location.

2.1 North Infiltration Pond – Alternative High School Building

Boring B-8 and test pit IT-2 (PIT excavation) were advanced in the east side of the infiltration pond, and boring OW-3 and test pit IT-3 were advanced in or near the southwest corner of the pond. Boring B-8 and test pit IT-2 encountered 1 to 2 feet of fill over Sandy Glacial Till. The Sandy Glacial Till was composed of medium dense to very dense silty sand or gravelly silty sand. Underlying the Sandy Glacial Till was Advance Outwash at a depth of 8.5 feet below ground surface (bgs) (elevation 118.5 feet). The Advance Outwash was composed of dense to very dense sand with interbedded hard silt or very dense silty sand lenses. OW-3 and IT-3 encountered 7 to 8.5 feet of fill over Sandy Glacial

Till composed of dense and very dense silty sand. Boring OW-3 extended past the Sandy Glacial Till into the Advance Outwash at 13 feet bgs. The Advance Outwash was composed of dense sand containing some interbedded lenses of gravel and silt.

2.2 Central Infiltration Pond – Science Building

Test pit TP-1 and IT-4 (PIT excavation) were advanced in the pond footprint and encountered 4 to 5 feet of fill over Advance Outwash. The Advance Outwash was composed of dense to very dense sand and gravel with interbedded lenses of silty sand. Boring OW-1, advanced near the pond west side, encountered 14 feet of fill. Underlying the fill was Advance Outwash composed of dense to very dense sand to the extent of the boring. Four borings (B-1 through B-4) were previously advanced in 1994 in the pond vicinity, and boring B-11 was advanced east of the pond southeast corner in August 2016. These previous explorations confirm our recent findings and correlate with the soil units we encountered. The previous borings indicate that the thickness of fill varies across the site and that Glacial Till with increased fines content are present at the east and southeast area of the site.

2.3 South Infiltration Pond – High School Staff Parking Lot

Borings B-15 and OW-2 and test pit IT-1 (PIT excavation) encountered 1 to 3.5 feet of fill. Underlying the fill was Advance Outwash composed of dense to very dense sand with interbedded lenses of silty sand. In boring B-15, no silty sand lenses were encountered, indicating the silty sand lenses encountered in IT-1 and OW-2 were isolated deposits.

2.4 Laboratory Test Results

Geotechnical laboratory test results indicate the Sandy Glacial Till has a fines content of 21 to 27 percent, and the Advance Outwash fines content varied between 1 and 18 percent. Table 1 summarizes the geotechnical testing results. Boring logs and test pit logs are included in Appendix A, and complete grain size distribution test results are presented in Appendix B. The boring logs and lab test results provide a detailed description of the soil strata. The soil stratigraphy adjacent to the three proposed infiltration pond locations is shown on cross-section A-A' (Figure 3).

At each PIT location (IT-1 through IT-4), a sample was collected of the soil underlying the base of each designed infiltration pond after completing our infiltration testing. The samples were submitted for laboratory analysis of organic content and cation exchange capacity. The laboratory results are summarized in Table 2.

Table 1 Summary of Geotechnical Laboratory Test Results

Pond Location	Exploration	Sample	Depth (feet)	Soil Type	Gravel Content (%)	Sand Content (%)	Fines Content (%)	D-10 (mm)
North Pond	IT-2	G-1	9	Outwash	1	96	3	0.14
North Pond	IT-2	G-3	12.5	Silt Lens	4	25	72	0.003*
North Pond	IT-3	G-1	9	Sandy glacial till	16	60	23	0.02*
North Pond	IT-3	G-2	11.25	Sandy glacial till	14	65	21	0.03*
North Pond	B-8	S-3	11.5	Outwash	19	66	15	0.01*
North Pond	OW-3	S-2	10	Sandy glacial till	24	50	27	0.05*
North Pond	OW-3	S-4	20	Sandy glacial till	2	77	21	0.04*
North Pond	OW-3	S-6	30	Sandy glacial till	—	—	24	—
Central Pond	IT-4	G-4	3	Outwash	61	39	1	0.31
Central Pond	IT-4	G-7	13	Outwash	36	61	3	0.10
Central Pond	OW-1	S-3	16	Outwash	4	86	10	0.07*
Central Pond	OW-1	S-6	30	Outwash	7	88	5	0.12
South Pond	IT-1	G-1	7	Outwash	7	92	2	0.17
South Pond	IT-1	G-2	9.5	Outwash	—	—	18	—
South Pond	IT-1	G-3	11.5	Outwash	28	64	8	0.09
South Pond	B-15	S-4A	15.5	Outwash	4	85	11	0.06*
South Pond	B-15	S-5	21.5	Outwash	—	—	14	—
South Pond	OW-2	S-2B	11	Silt Lens	2	55	43	0.01*
South Pond	OW-2	S-3	15	Outwash	10	82	8	0.11

— = Not Tested

* = Amec Foster Wheeler estimated values based on extrapolation of laboratory data.

Table 2 Organic Content and Cation Exchange Capacity Test Results

Exploration	Sample	Pond Location	Depth (feet)	Organic Content (%)*	Cation Exchange Capacity (meq/100g)**
IT-2	G-2	North Pond – East Side	-11.5	0.5	1.9
IT-3	G-2	North Pond – West Side	-11.25	1.1	6.7
IT-4	G-1	Central Pond	-9.0	0.7	1.2
IT-1	G-2	South Pond	-9.5	0.7	2.6

* = Percent by weight.

** meq/100g = milliequivalents per 100 grams.

Test results for organic content met the Kitsap County Stormwater Manual minimum requirements of 0.5 percent. Cation exchange capacity testing results shows only sample IT-3/G-2 at the north infiltration pond southwest corner met the Kitsap County Stormwater Manual minimum requirement of 5 meq/100g.

3.0 GROUNDWATER CONDITIONS

During our preliminary geotechnical exploration in August 2016, only boring B-15 at the south infiltration pond encountered groundwater. Groundwater was encountered in boring B-15 at 18 feet bgs (elevation 91). To collect seasonal groundwater levels, three observation wells (OW-1 through OW-3) were installed near the infiltration pond locations on December 7, 2016.

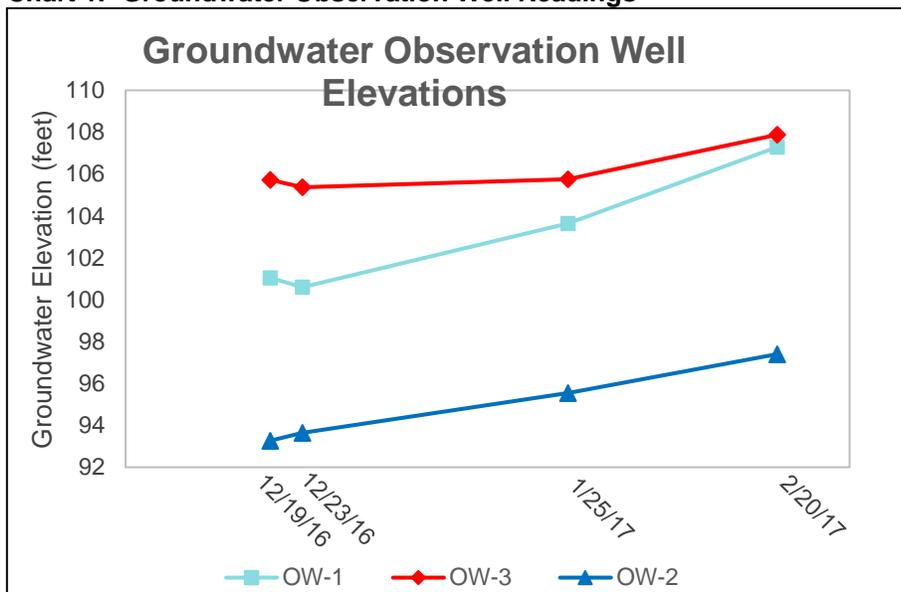
- Boring OW-3, at the north infiltration pond, encountered groundwater at 30 feet bgs (elevation 97).
- Boring OW-1, at the central infiltration pond, encountered groundwater 25 feet bgs (elevation 96).
- Boring OW-2, at the south infiltration pond, did not encounter groundwater at the time of drilling.

Within all three borings, mottling was noted on the native soil directly underlying the fill. We interpret the mottling to originate from groundwater infiltrating through the less dense fill soils, resting on the native soil horizon, and then infiltrating at a slower rate. Test Pit IT-4 at the central infiltration pond encountered groundwater seepage in the north sidewall at 7 to 8.75 feet bgs. The seepage appeared to originate from groundwater resting and infiltrating at a slower rate on the two very dense silty sand layers noted at 7.5 feet and 8.75 bgs.

From December 7, 2016, to February 20, 2017, five periodic groundwater readings were taken from OW-1 through OW-3. Based on the groundwater readings and subsurface exploration logs, the groundwater gradient at the site appears to be toward the south to southwest.

The groundwater data collected indicates 3 feet or more feet of vertical separation between the infiltration pond design base elevations and groundwater. The highest groundwater elevation readings and inferred groundwater gradient are displayed on Figure 3. Chart 1 summarizes our collected water level readings.

Chart 1. Groundwater Observation Well Readings



4.0 STEEP SLOPE EVALUATION

As a general guideline to prevent seepage on the face of slopes, the ponds should be set back from the top of steep slopes a horizontal distance equal to the vertical height of the slope, as referenced in the Kitsap County Stormwater Manual (Section 7.3.4.1.N). This guideline is appropriate for slopes that are 30 percent or flatter. Additionally, the level bench at the top of the slope should have a minimum width of 6 feet, but may need to be wider to provide an adequate setback distance.

Directly to the west and southwest of the north infiltration pond, the existing slope dips downward toward NW Anderson Hill Road, with a maximum slope of 55 percent for the first 10 vertical feet, and thereafter a reduced slope of approximately 11 percent. Geologic Cross-Section B-B' (Figure 4) illustrates the slope at the infiltration pond southwest corner. Although slope stability is not a concern, there is a risk of water infiltrating from the pond and seeping out onto the face of the slope. For this reason, we recommend constructing a fill embankment on the slope in order to increase the separation between the pond and the face of the slope, and flatten the slope. We understand that AHBL has revised the grading plans in this manner to mitigate the risk of seepage on the slope.

At the south side of the central pond, a steep cut slope is proposed between the pond and the regraded access drive. We understand that AHBL revised the pond location and grading according to these recommendations to mitigate the risk of seepage on the slope.

If utility trenches descend steep slopes, such as pond outfall pipes, anti-seepage diaphragms should be installed to reduce seepage through the trench backfill. The anti-seepage diaphragms should consist of concrete or controlled density fill that is at least 1 foot thick (in line with the pipe) and extends at least 1 foot beyond the trench bottom and sidewalls. We recommend installing at least one anti-seepage diaphragm every 100 feet along a trench that is on a steep slope (greater than 15 percent per Kitsap County requirements).

5.0 INFILTRATION TESTING METHOD

We conducted four PITs, designated IT-1 through IT-4, at the proposed location of the infiltration ponds at the time of testing. We conducted IT-1 in the south infiltration pond footprint and IT-2 and IT-3 in the north infiltration pond footprint from December 21 through 23, 2016. When the central infiltration pond was added to the infiltration facility, we completed PIT IT-4 in the infiltration pond footprint on February 20, 2017.

Our large-scale infiltration tests were performed in general accordance with the PIT methodology described in the *Kitsap County Stormwater Design Manual*, Chapter 7.3 and Appendix 7-B (Kitsap County, 2010). The *Kitsap County Stormwater Design Manual* also references testing procedures from the Washington State Department of Ecology (Ecology, 2005). As discussed in both referenced stormwater manuals, large-scale infiltration testing provides a more representative indicator of infiltration facility performance than smaller test methods. However, because of the large area required to perform a PIT, coupled with site restrictions and existing structures, we selected the large-diameter, single-ring percolation PIT to determine site infiltration rates. Prior to starting in situ infiltration testing (IT-1 through IT-4), AHBL provided the infiltration pond locations and design infiltration elevations. Our PIT testing method is described in Appendix C.

6.0 DESIGN INFILTRATION RATES AND TESTING DATA

Amec Foster Wheeler calculated the long-term design infiltration rates for each of the four infiltration ponds using in situ infiltration testing data following methods specified in the 2010 Kitsap County Stormwater Manual, Section 7.3.4.1.C “General Requirements for Infiltration Facilities – Design Infiltration Rates” and design guidelines specified in Chapters 3.3.5 and 3.3.9 of Volume III of the 2005 Department of Ecology *Stormwater Manual for Western Washington* (Ecology, 2005). The correction factors and long-term design infiltration rates are tabulated in Table 3.

After compiling and analyzing the collected field data, the infiltration rates for each pond location were calculated. Plots of the measured infiltration rates over the duration of testing for the four PIT are presented in Charts 2 through 5.

Chart 2 North Infiltration Pond (East Side) Measured Infiltration Rate

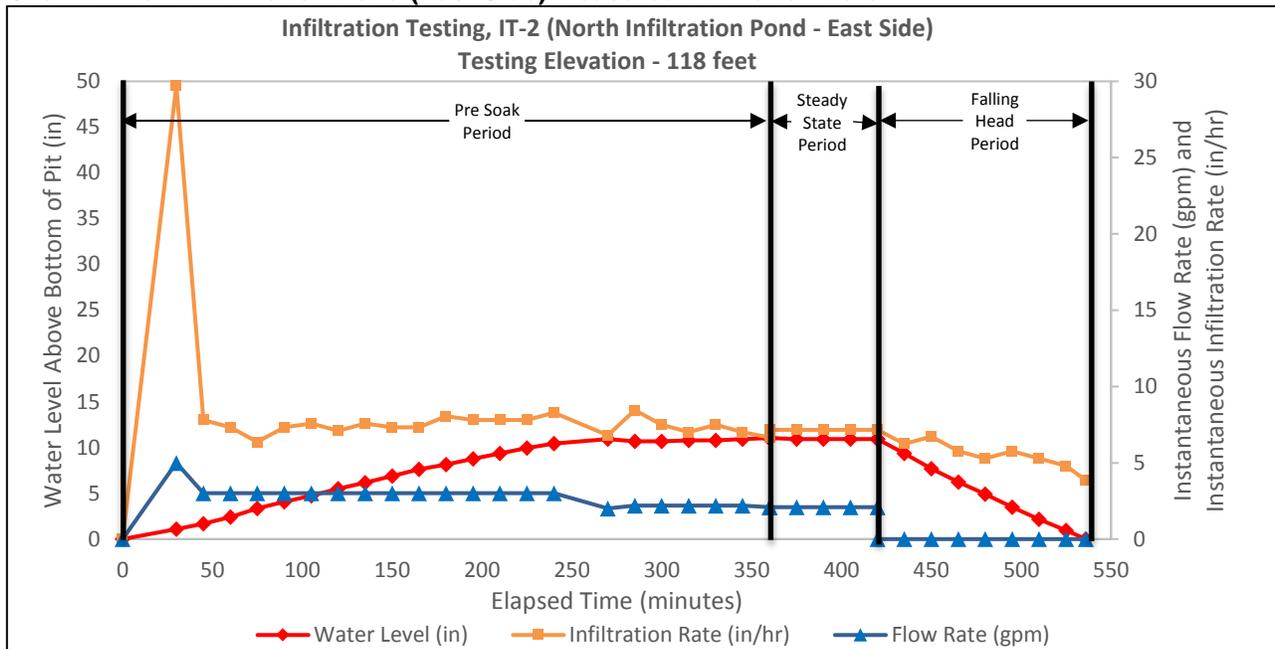
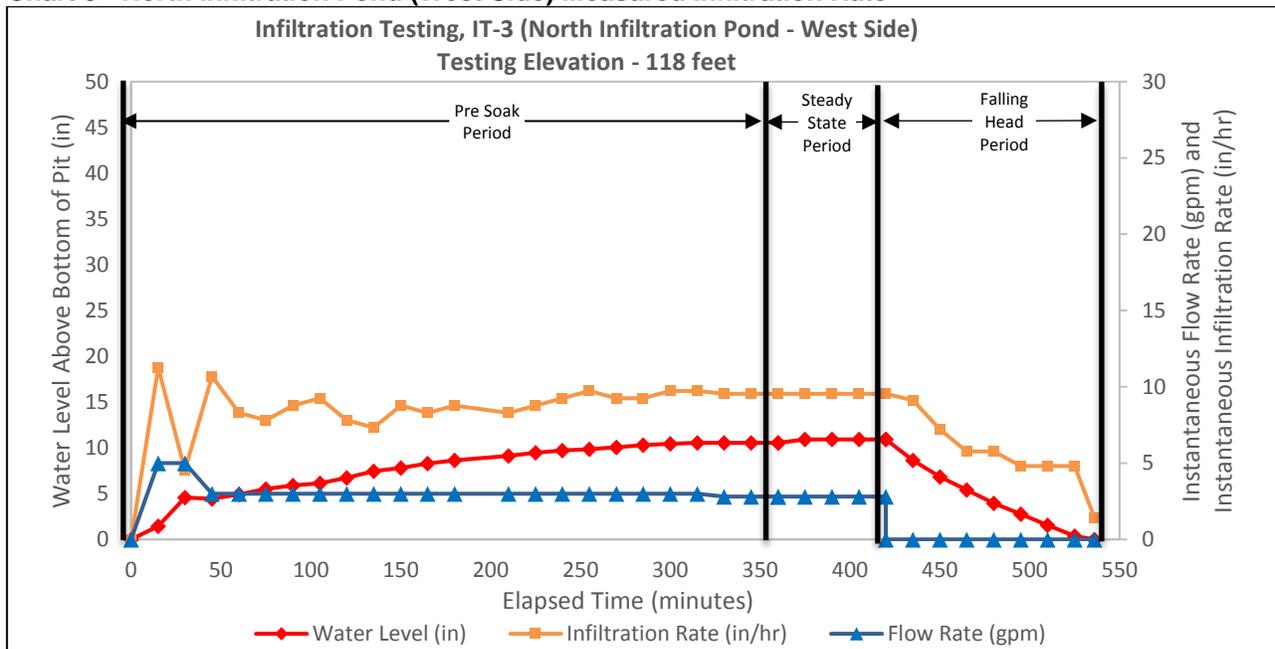


Chart 3 North Infiltration Pond (West Side) Measured Infiltration Rate



The constant-head (steady-state period) measured infiltration rates in the north infiltration pond were 7.1 inches/hour (IT-2) and 9.2 inches/hour (IT-3), for an average rate of 8.1 inches/hour. Using an intermediate correction factor for site variability, due to silt lenses, the design infiltration rate for the north infiltration pond was 0.8 inch per hour, after applying correction factors shown in Table 3.

Chart 4 Central Infiltration Pond Measured Infiltration Rate

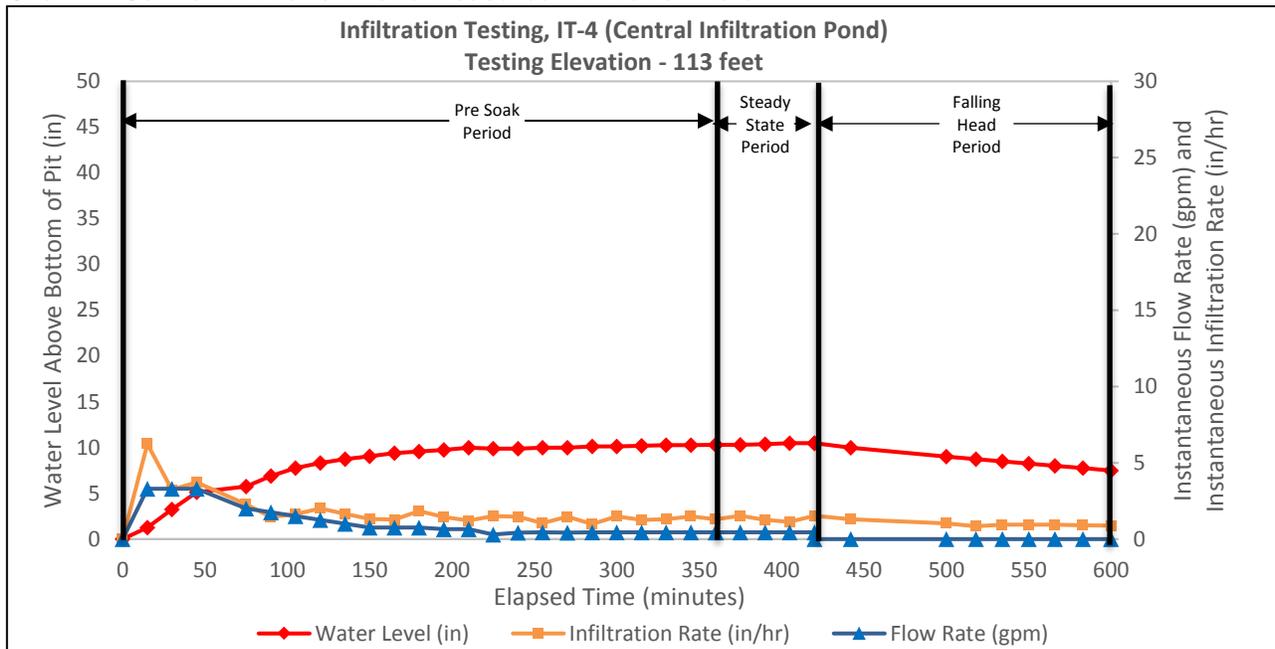
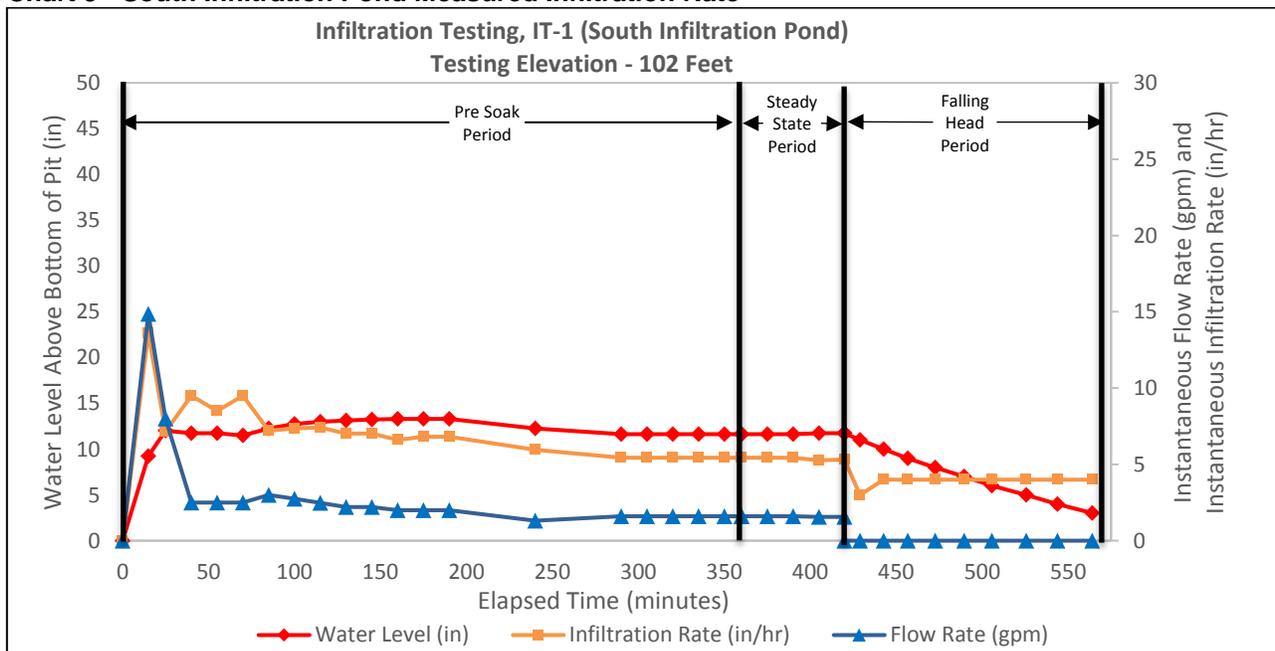


Chart 5 South Infiltration Pond Measured Infiltration Rate



Using the constant-head (steady-state period) measured infiltration rate (1.3 inches per hour) and an intermediate correction factor for site variability, due to silt lenses, the design infiltration rate for the central infiltration pond was found to be 0.16 inch per hour, after applying correction factors shown in Table 3.

Using the constant-head (steady-state period) measured infiltration rate (5.4 inches per hour), the design infiltration rate for the south infiltration pond was found to be 0.8 inch per hour, after applying correction factors shown in Table 3.

Table 3 Correction Factors and Infiltration Design Rate

PIT	Pond Location	Measured constant head infiltration rate (inches/hour)	CF _v = correction factor for site variability	CF _m = correction factor for long-term maintenance	CF _i = correction factor for influent control	Total Correction Factor	Long-term Design Infiltration Rate (inches / hour)
IT-2 & IT-3	North Pond	8.1	3	3	2	8 (equivalent to 0.125)	1.0
IT-4	Central Pond	1.3	3	3	2	8 (equivalent to 0.125)	0.16
IT-1	South Pond	5.4	2	3	2	7 (equivalent to 0.84)	0.84

7.0 RECOMMENDED ADDITIONAL SERVICES

The *Kitsap County Stormwater Manual* (Section 7.3.4.1.E) requires performance testing of the completed infiltration facility. After the ponds have been excavated to subgrade elevation, Amec Foster Wheeler will confirm the subgrade soil types and work with the contractor to perform performance PITs.

8.0 LIMITATIONS

This report has been prepared for the exclusive use of Central Kitsap School District and their consultants for specific application to this project, in accordance with generally accepted geotechnical engineering and hydrogeologic practice. The infiltration recommendations presented in this report are based, in part, on the explorations Amec Foster Wheeler performed and used for this study and on information provided for the proposed project. Subsurface information obtained from borings, wells, test pits and PITs represents conditions at specific locations at the time of the observations and may not reflect conditions at other locations. If variations in subsurface conditions are observed at a later time, additional explorations may be needed and we may need to modify this report to reflect those

changes. We are available to provide geotechnical engineering throughout the design process, and construction and quality assurance monitoring during construction.

9.0 REFERENCES

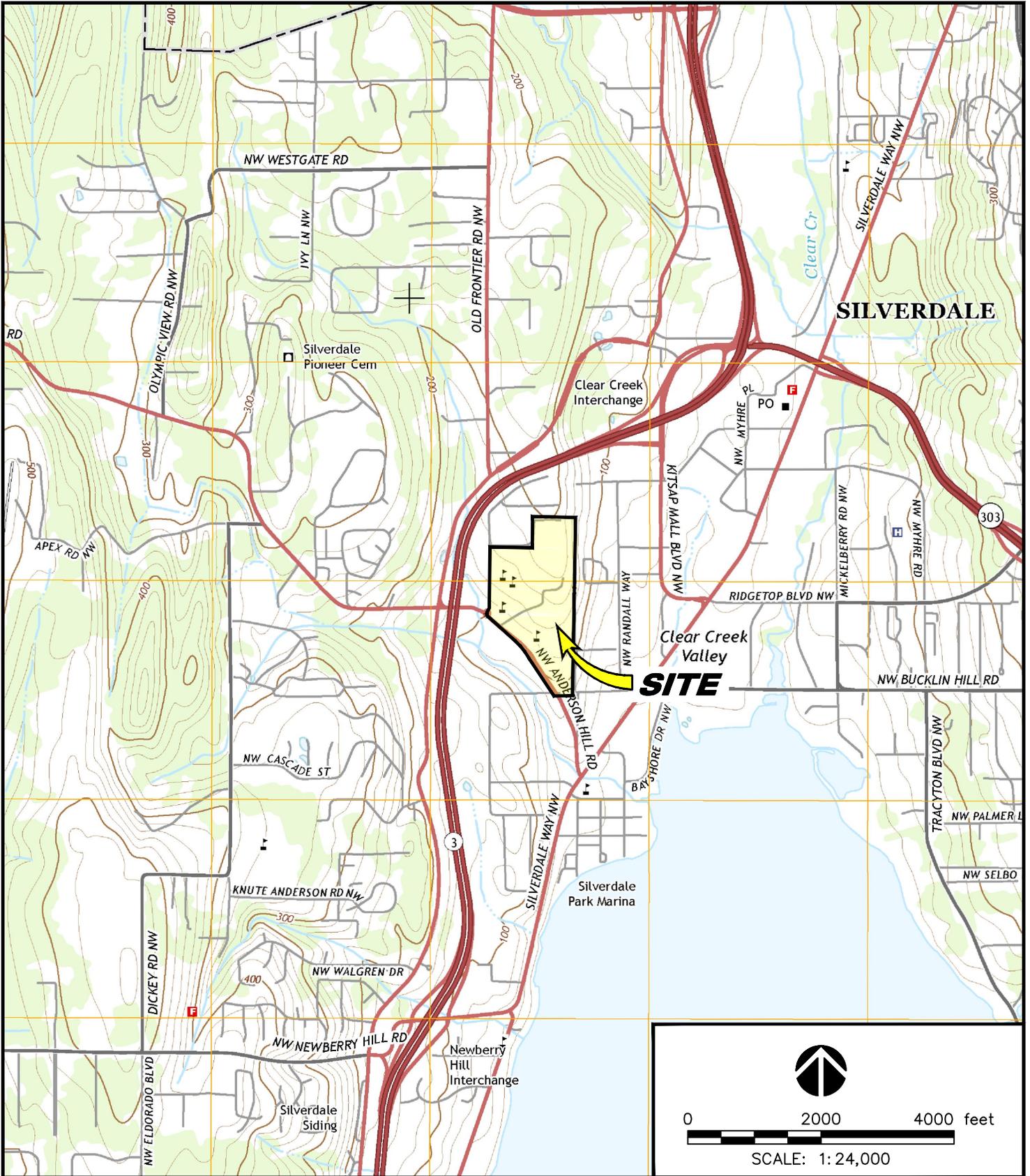
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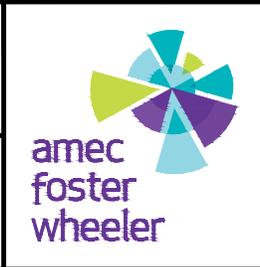
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FIGURES



CENTRAL KITSAP SCHOOL DISTRICT

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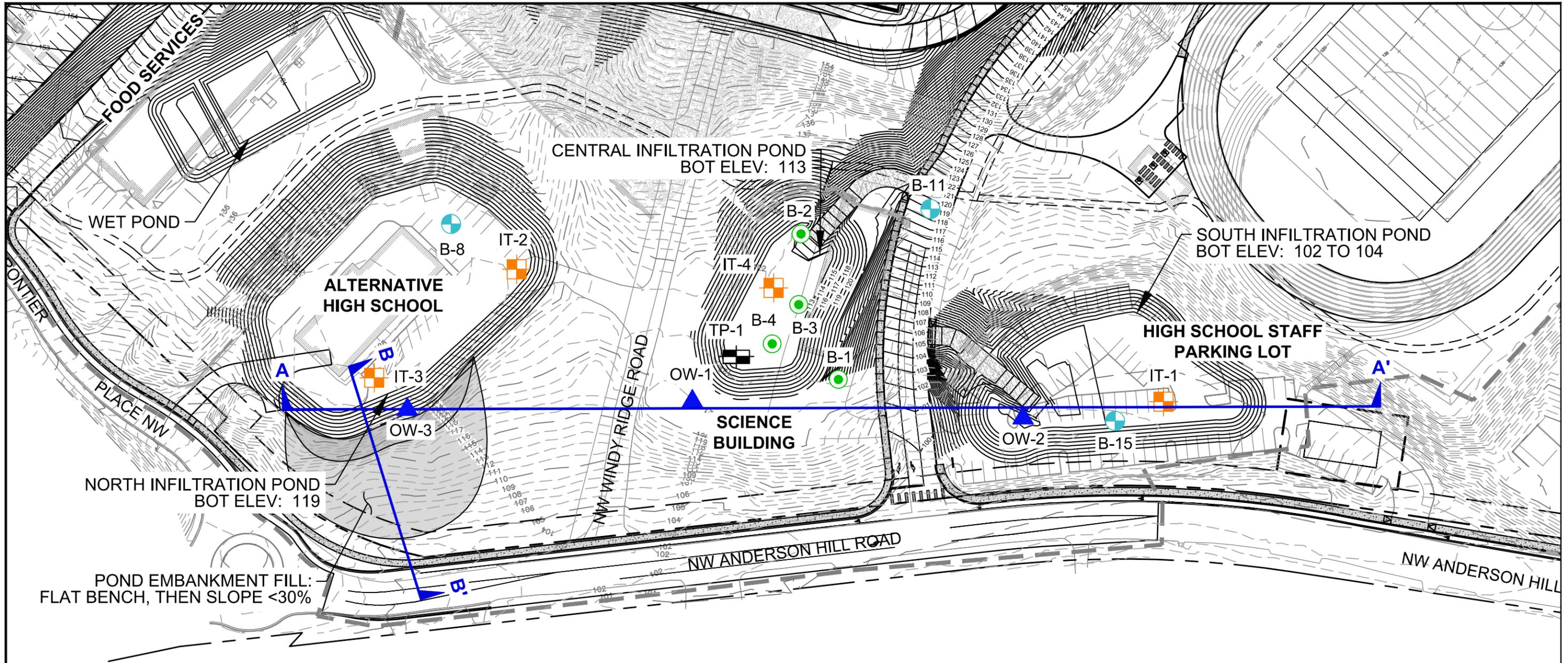


CENTRAL KITSAP HIGH SCHOOL
 AND MIDDLE SCHOOL CAMPUS

SITE LOCATION MAP

DATE	MARCH 2017
SCALE	1" = 2,000'
PROJECT NO.	6-917-18096-0
FIGURE	1

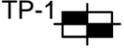
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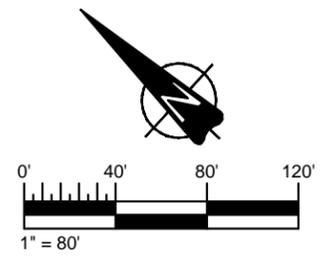


NOTE:

WE ESTIMATED THE RELATIVE LOCATION OF EACH EXPLORATION BY MEASURING FROM EXISTING FEATURES AND SCALING THESE MEASUREMENTS ONTO A LAYOUT PLAN SUPPLIED TO US. THE LOCATIONS DEPICTED ON THIS FIGURE SHOULD BE CONSIDERED ACCURATE ONLY TO THE DEGREE PERMITTED BY OUR DATA SOURCES AND IMPLIED BY OUR MEASURING METHODS.

LEGEND

- IT-4  INFILTRATION LOCATION
- OW-3  WELL LOCATION
- B-15  BORING NUMBER AND LOCATION
- TP-1  TEST PIT NUMBER AND LOCATION
- B-4  1994 BORING NUMBER AND LOCATION
- A-A'  CROSS-SECTION LOCATION



NOTE:
SURVEY BASE PROVIDED BY AHBL, DATED MARCH 2nd, 2017.

CENTRAL KITSAP SCHOOL DISTRICT

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Environment & Infrastructure, Inc.
11810 North Creek Parkway North
Bothell, WA 98011

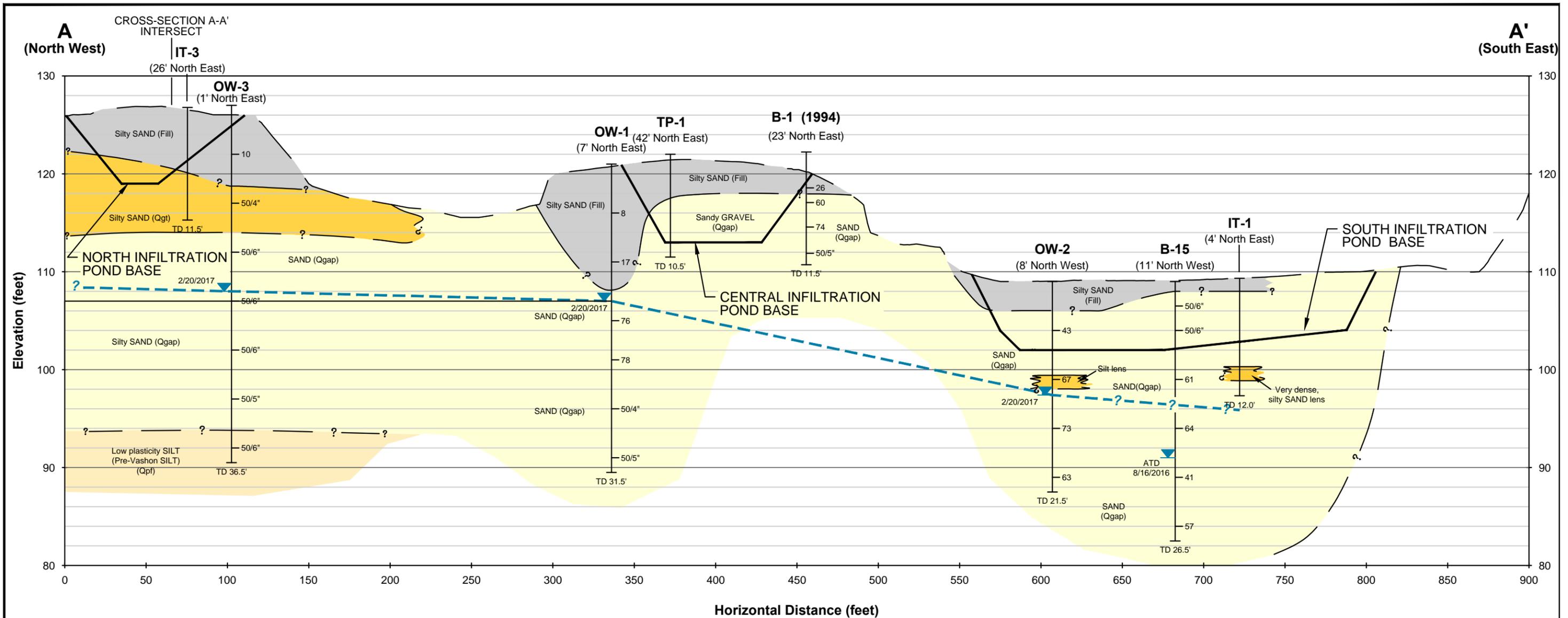


CENTRAL KITSAP HIGH SCHOOL AND
MIDDLE SCHOOL CAMPUS

SITE AND EXPLORATION PLAN

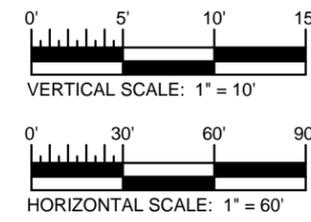
DATE	MARCH 2017
SCALE	1" = 80'
PROJECT NO.	6-917-18096-0
FIGURE	2

DRAWN BY: JRS CHECKED BY: _____



LEGEND

- OW-3**
(1' North)
 - IT-3**
(26' North East)
 - OW-1**
(7' North East)
 - TP-1**
(42' North East)
 - B-1 (1994)**
(23' North East)
 - OW-2**
(8' North West)
 - B-15**
(11' North West)
 - IT-1**
(4' North East)
- STANDARD PENETRATION RESISTANCE IN BLOWS PER FOOT
- GROUNDWATER LEVEL - HIGHEST ELEVATION DURING WINTER SEASON OR AT TIME OF DRILLING
- TOTAL DEPTH OF BORING



NOTES:

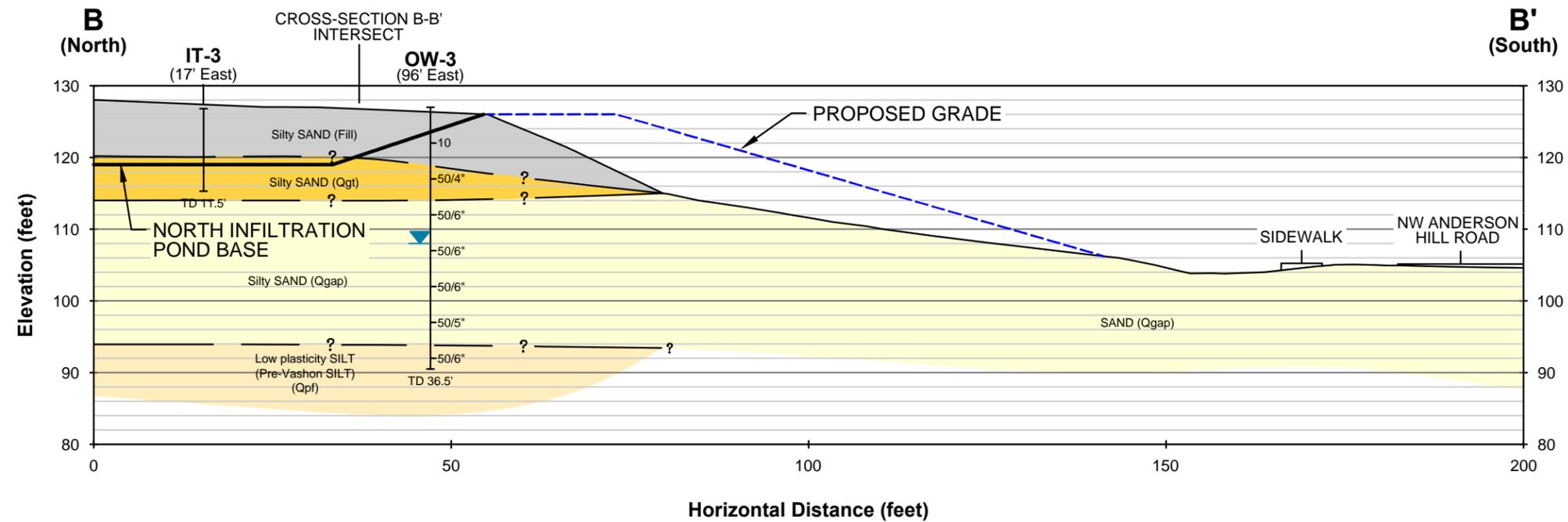
THE STRATA ARE BASED UPON INTERPOLATION BETWEEN EXPLORATIONS AND MAY NOT REPRESENT ACTUAL SUBSURFACE CONDITIONS.

SIMPLIFIED NAMES ARE SHOWN FOR SOIL DEPOSITS, BASED ON GENERALIZATIONS OF SOIL DESCRIPTIONS. SEE EXPLORATION LOGS AND REPORT TEXT FOR COMPLETE SOIL DESCRIPTIONS.

DRAWN BY: JRS CHECKED BY: KB

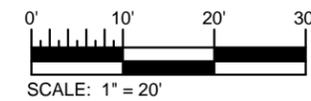
G:\911\18000\18096 - CKMS\18096-11.dwg - Geology A - Mar. 09, 2017 11:29pm - jeffrey.sanders

CENTRAL KITSAP SCHOOL DISTRICT Amec Foster Wheeler Environment & Infrastructure, Inc. 11810 North Creek Parkway North Bothell, WA 98011		CENTRAL KITSAP HIGH SCHOOL AND MIDDLE SCHOOL CAMPUS	DATE MARCH 2017
		GEOLOGIC CROSS-SECTION A-A'	SCALE 1" = 60' PROJECT NO. 6-917-18096-0 FIGURE 3



LEGEND

- OW-3** (96' East) BORING NUMBER AND APPROXIMATE LOCATION OFFSET FROM SECTION LINE
- STANDARD PENETRATION RESISTANCE IN BLOWS PER FOOT
- GROUNDWATER LEVEL - HIGHEST ELEVATION DURING WINTER SEASON OR AT TIME OF DRILLING
- TOTAL DEPTH OF BORING



NOTES:

THE STRATA ARE BASED UPON INTERPOLATION BETWEEN EXPLORATIONS AND MAY NOT REPRESENT ACTUAL SUBSURFACE CONDITIONS.

SIMPLIFIED NAMES ARE SHOWN FOR SOIL DEPOSITS, BASED ON GENERALIZATIONS OF SOIL DESCRIPTIONS. SEE EXPLORATION LOGS AND REPORT TEXT FOR COMPLETE SOIL DESCRIPTIONS.

DRAWN BY: JRS CHECKED BY: KB

CENTRAL KITSAP SCHOOL DISTRICT Amec Foster Wheeler Environment & Infrastructure, Inc. 11810 North Creek Parkway North Bothell, WA 98011		CENTRAL KITSAP HIGH SCHOOL AND MIDDLE SCHOOL CAMPUS	DATE MARCH 2017
		GEOLOGIC CROSS-SECTION B-B'	SCALE 1" = 60' PROJECT NO. 6-917-18096-0 FIGURE 4



APPENDIX A

Field Exploration Procedures and Logs

APPENDIX A

FIELD EXPLORATION PROCEDURES AND LOGS

Central Kitsap High School and Middle School Redevelopment
Silverdale, Washington
Project No. 6-917-18096-0

The following paragraphs describe the procedures used for field explorations and field tests that Amec Foster Wheeler conducted for this project. Descriptive logs of our explorations are enclosed in this appendix.

AUGER BORING PROCEDURES

Exploratory borings were advanced with a hollow-stem auger, using a trailer-mounted drill rig operated by an independent drilling firm working under subcontract to Amec Foster Wheeler. An engineering geologist from Amec Foster Wheeler continuously observed the borings, logged the subsurface conditions, and collected representative soil samples. All samples were stored in watertight containers and later transported to the laboratory for further visual examination and testing. After each boring was completed, the borehole was backfilled with a mixture of bentonite chips and soil cuttings, and the surface was patched with asphalt or concrete (where appropriate).

Throughout the drilling operation, soil samples were obtained at 2.5- or 5-foot depth intervals by means of the standard penetration test (SPT) per ASTM D-1586. This testing and sampling procedure consists of driving a standard 2-inch-diameter steel split-spoon sampler 18 inches into the soil with a 140-pound hammer free-falling 30 inches. The number of blows required to drive the sampler through each 6-inch interval was counted, and the total number of blows struck during the final 12 inches was recorded as the standard penetration resistance, or “SPT blow count.” If a total of 50 blows were struck within any 6-inch interval, the driving was stopped and the blow count was recorded as 50 blows for the actual penetration distance. The resulting standard penetration resistance values indicate the relative density of granular soils and the relative consistency of cohesive soils.

The enclosed boring logs describe the vertical sequence of soils and materials encountered in each boring, based primarily on field classifications and supported by subsequent laboratory examination and testing. Where a soil contact was observed to be gradational, boring logs indicate the average contact depth. Where a soil type changed between sample intervals, we inferred the contact depth. The boring logs also graphically indicate the blow count, sample type, sample number, and approximate depth of each soil sample obtained from the borings, as well as any laboratory tests performed on these soil samples. If any groundwater was encountered in a borehole, the approximate

groundwater depth is depicted on the boring log. Groundwater depth estimates are typically based on the moisture content of soil samples, the wetted height on the drilling rods, and the water level measured in the borehole after the auger has been extracted.

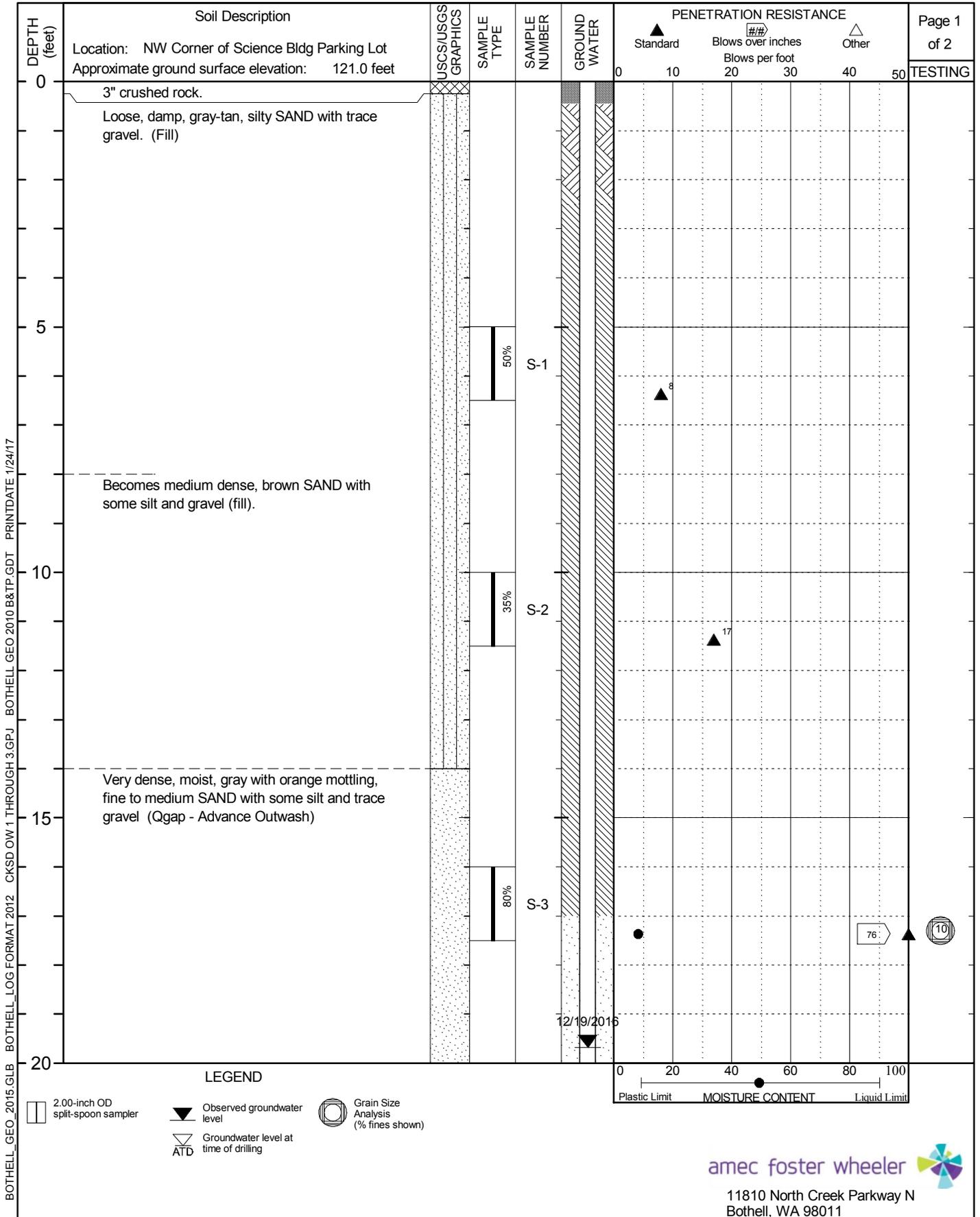
WELL INSTALLATION PROCEDURES

Our groundwater observation wells consist of 1.5-inch-diameter polyvinyl chloride pipe, the lower 5 feet of which is finely slotted. The annular space around the slotted segment was backfilled with clean sand and gravel, and the upper portion of annulus was sealed with bentonite chips and concrete. A flush-mounted monument was placed over the top of each wellhead for protection. The as-built configuration of each observation well is illustrated on the respective *Boring Log*. Our logs also show any groundwater levels encountered at the time of our drilling.

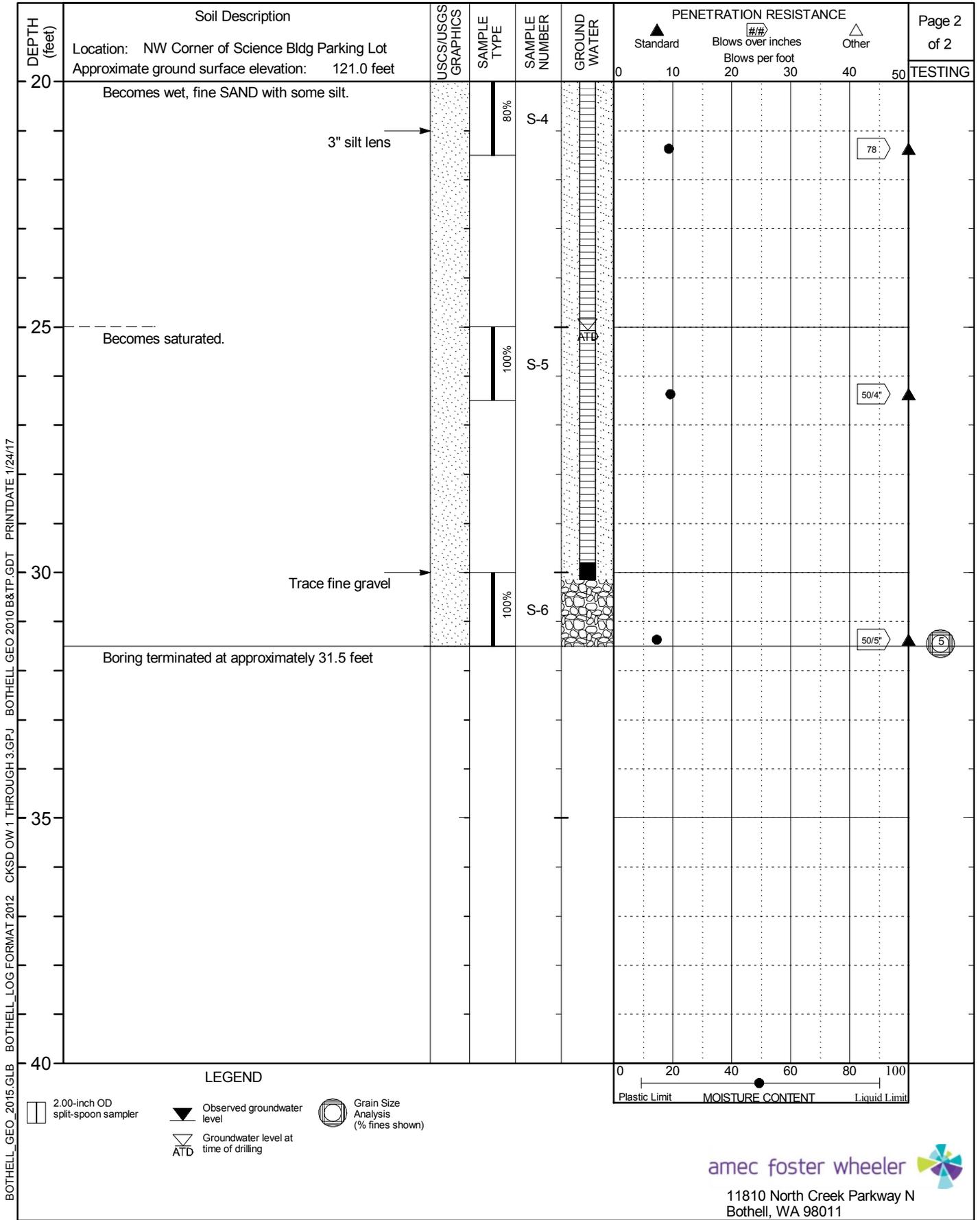
TEST PIT PROCEDURES

Our exploratory test pits were excavated with a track-mounted hoe operated by an independent firm working under subcontract to Amec Foster Wheeler. A geotechnical specialist from our firm continuously observed the test pit excavations, logged the subsurface conditions, and obtained representative soil samples. All samples were stored in watertight containers and later transported to a laboratory for further visual examination and testing. After we logged each test pit, the hoe operator backfilled it in lifts and compacted each lift to a firm or firm and unyielding condition. The upper 1 foot was backfilled with crushed rock and compacted to the firm/unyielding condition. Approximately 4 inches of asphalt was placed at the surface for any test pits excavated within paved areas.

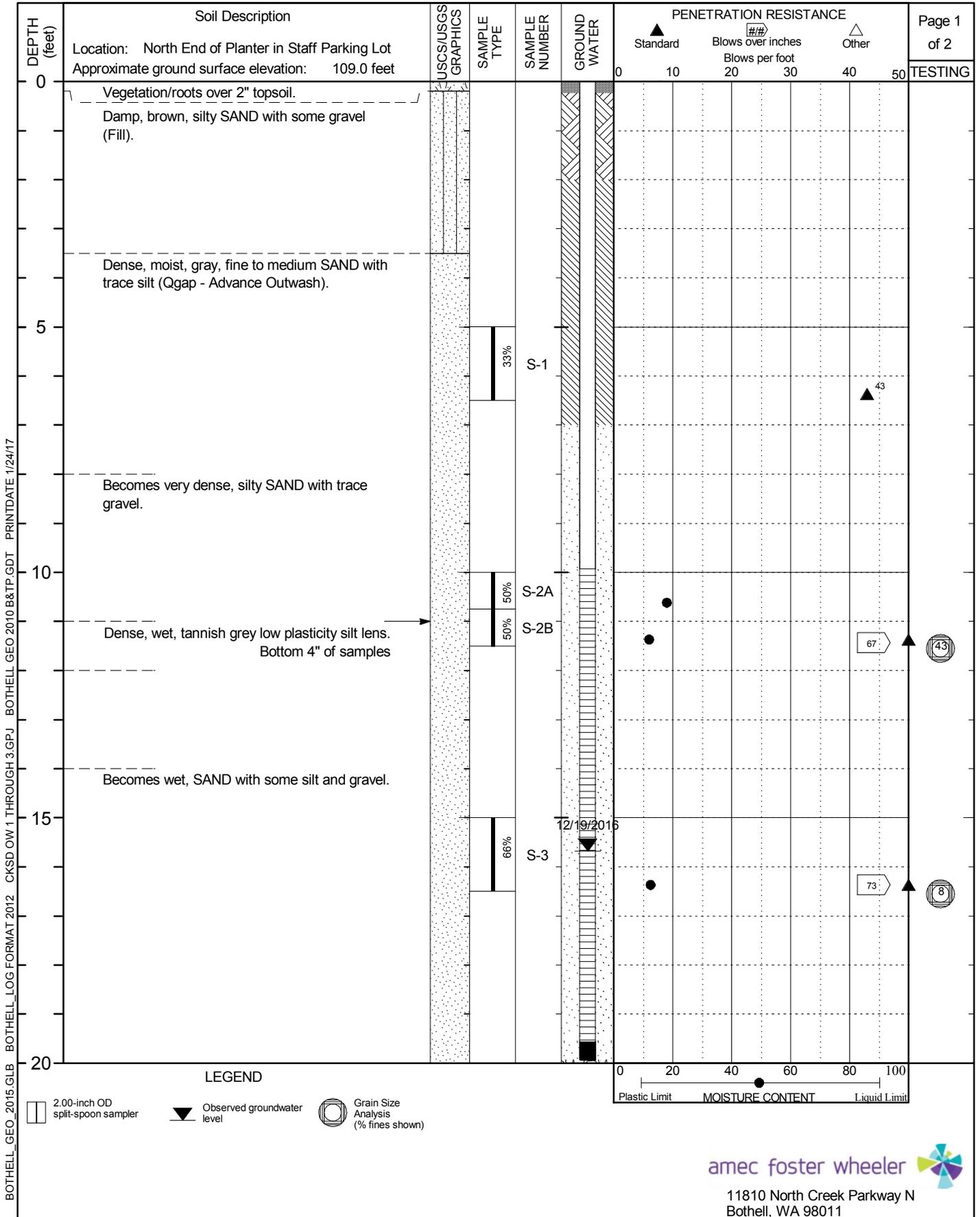
The enclosed *Test Pit Logs* indicate the vertical sequence of soils and materials encountered in each test pit, based primarily on our field classifications and supported by subsequent laboratory examination and testing. Where a soil contact was observed to be gradational or undulating, our logs indicate the average contact depth. We estimated the relative density and consistency of the in situ soils by means of the excavation characteristics and the stability of the test pit sidewalls. Our logs also indicate the approximate depths of any sidewall caving or groundwater seepage observed in the test pits, as well as all sample numbers and sampling locations



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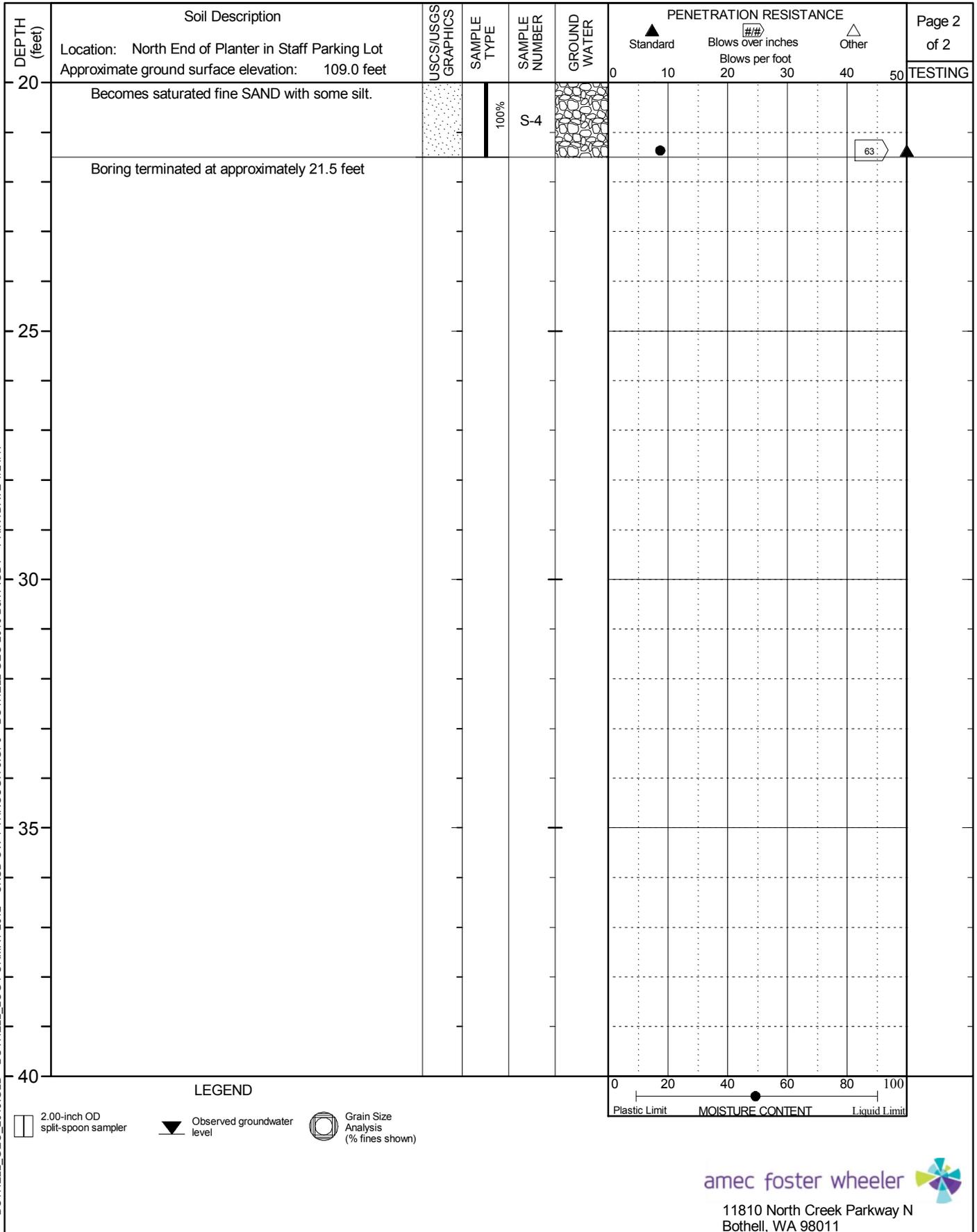


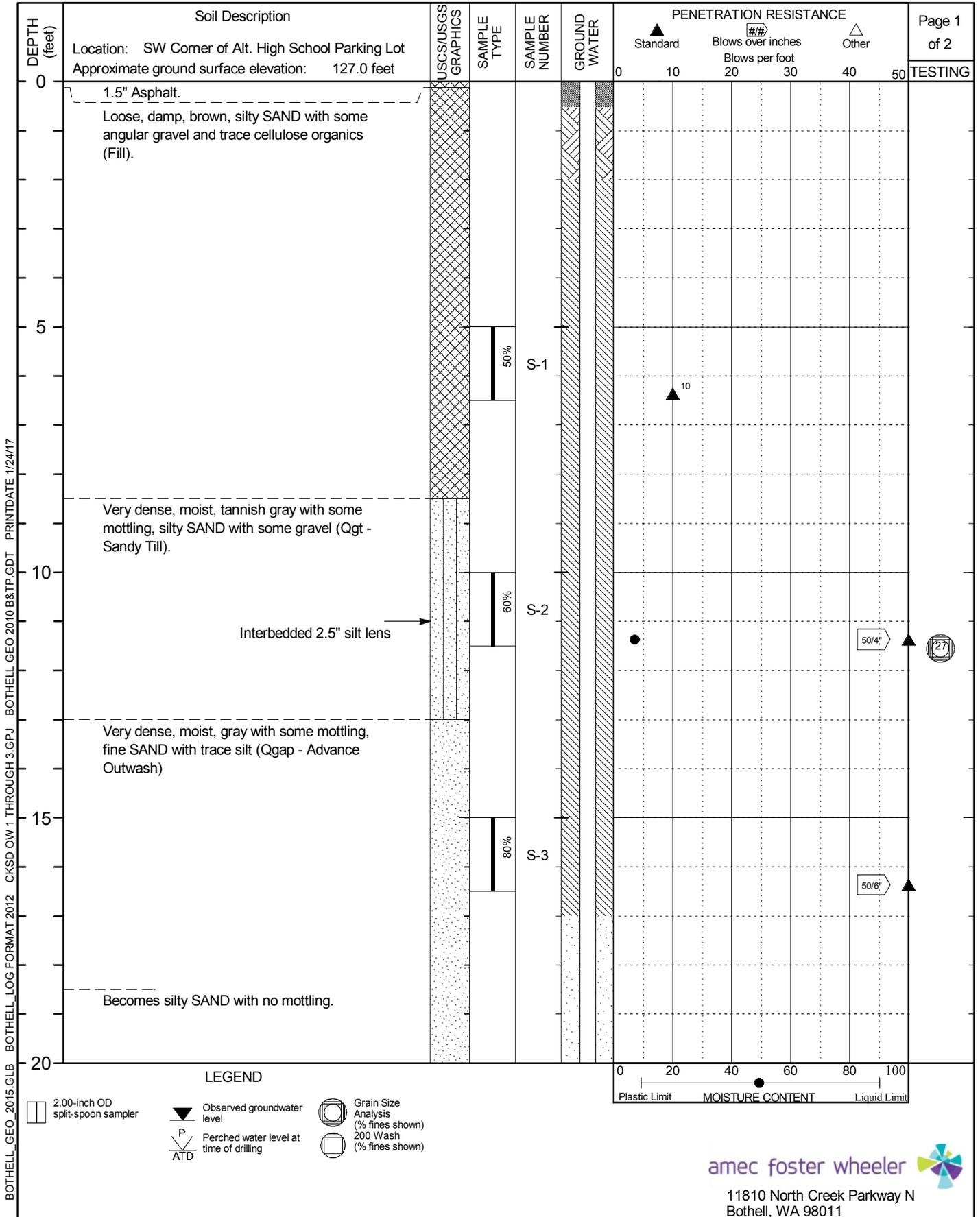
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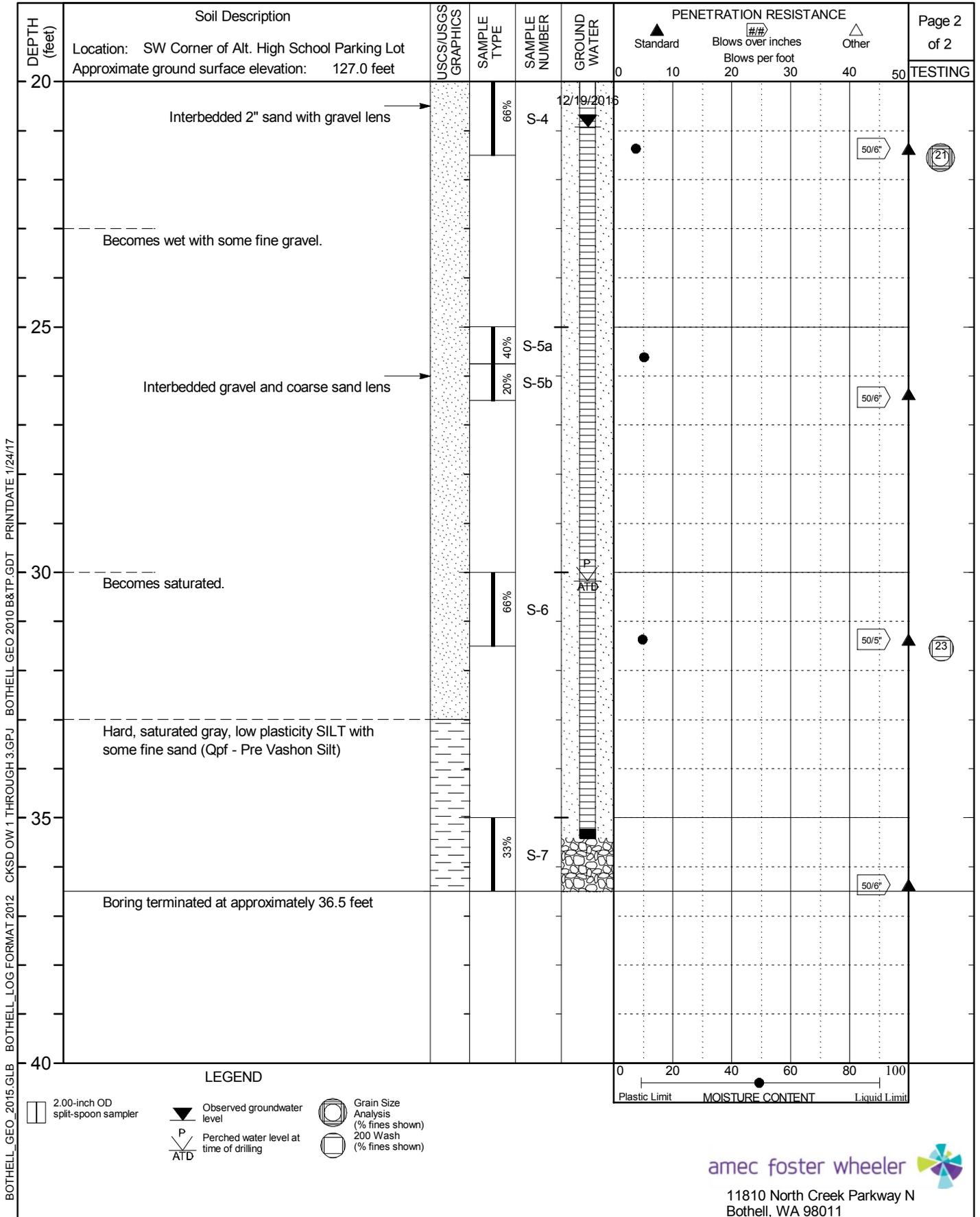
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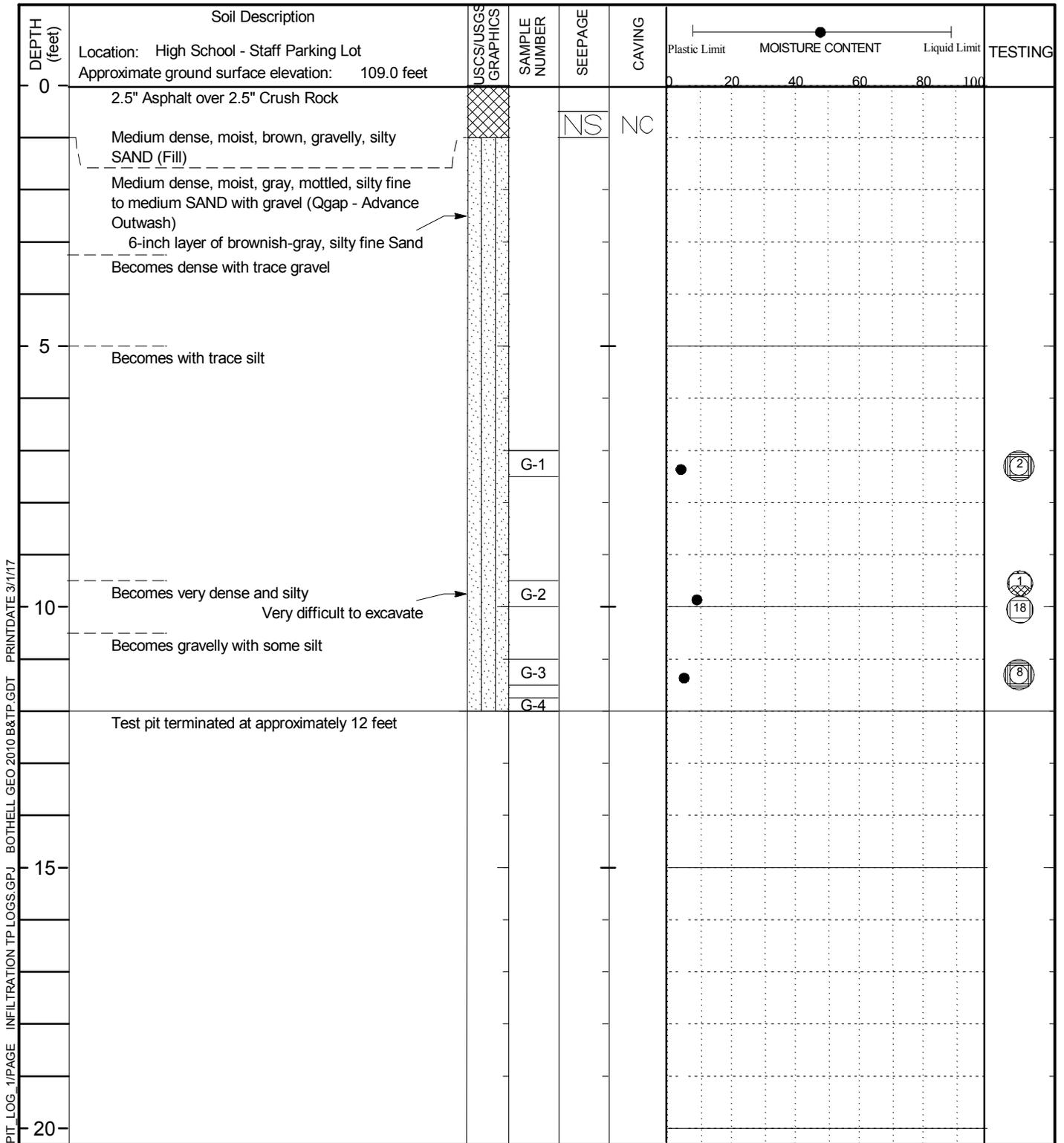


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BOTHELL_GEO_2015.GLB BOTHELL_LOG FORMAT 2012 CKSD OW 1 THROUGH 3.GPJ BOTHELL GEO 2010 B&TP.GDT PRINTDATE 1/24/17

Test Pit No.: IT-1



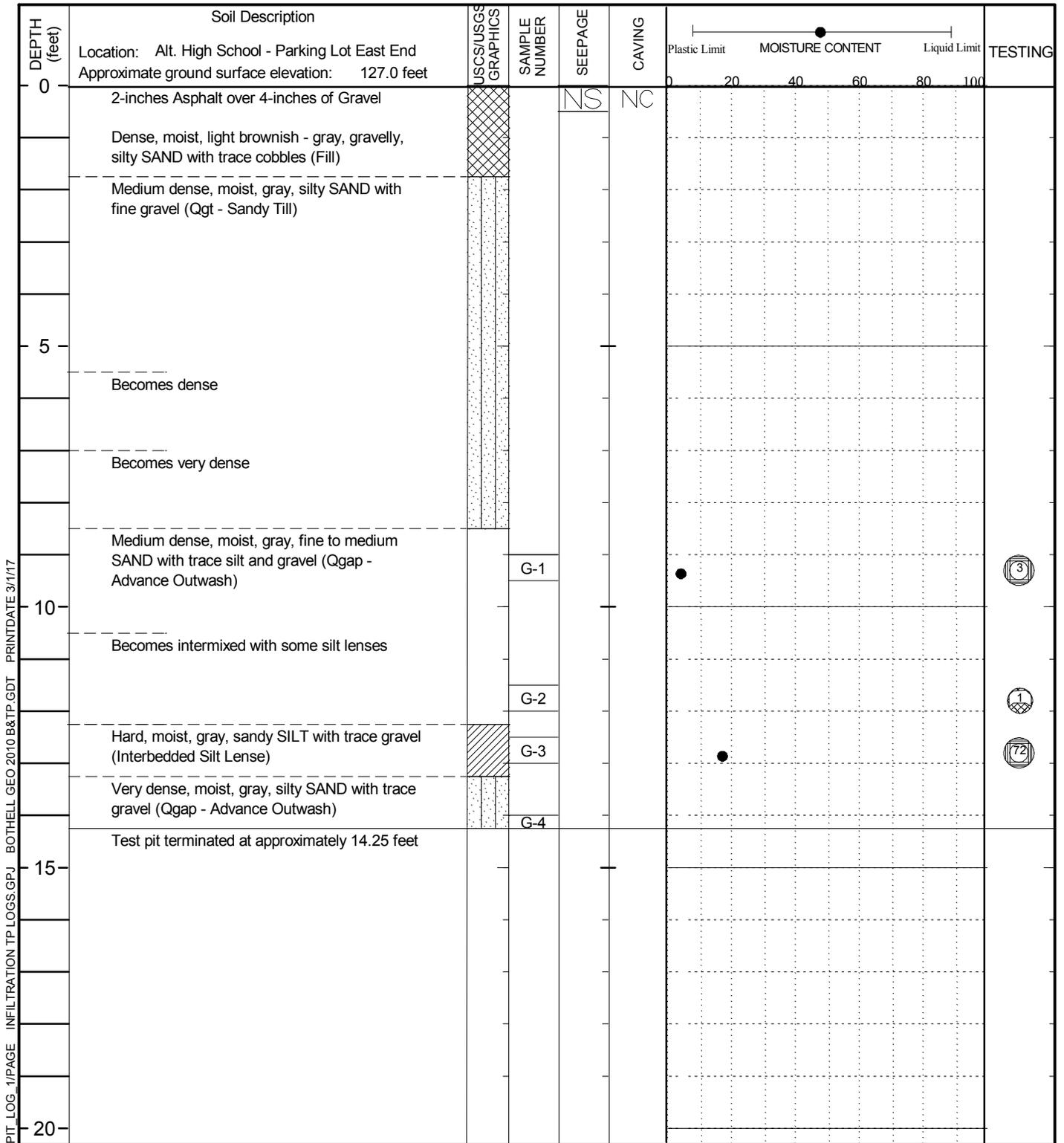
BOTHELL_GEO_2015.GLB TEST_PIT_LOG_1/PAGE INFILTRATION TP LOGS.GPJ BOTHELL_GEO 2010 B&TP.GDT PRINTDATE 3/1/17

LEGEND

- }} Moderate Seepage
- NC no caving observed
- NS no seepage observed
- [Square with dots] 200 Wash (% fines shown)
- [Square with horizontal lines] Grain Size Analysis (% fines shown)
- [Square with vertical lines] Organic Content (% shown)

Excavation Method: Trackhoe
 Date Excavated: December 21, 2016
 Logged By: KHM

Test Pit No.: IT-2



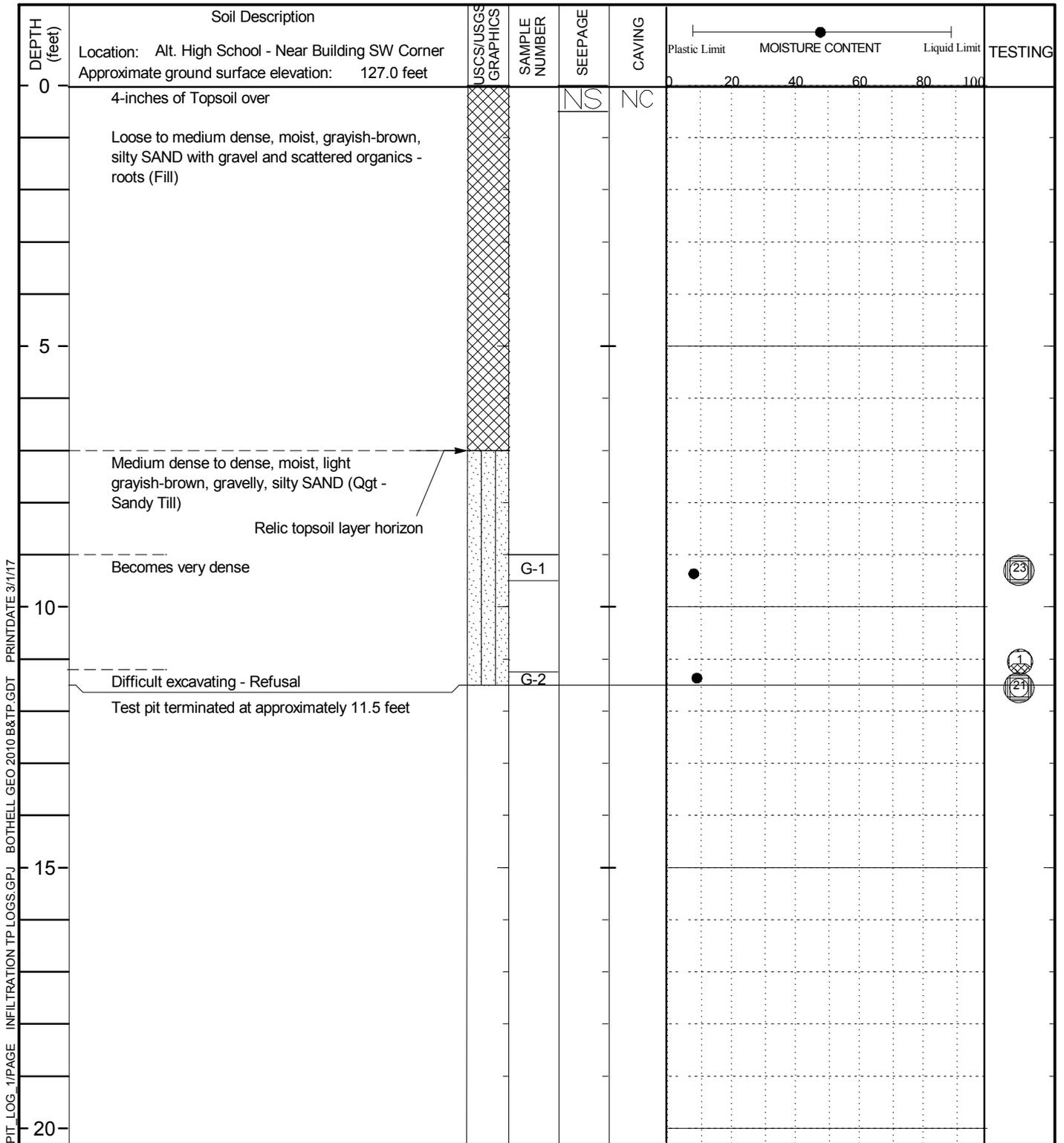
BOTHELL_GEO_2015.GLB TEST_PIT_LOG_1/PAGE INFILTRATION TP LOGS.GPJ BOTHELL_GEO 2010 B&TP.GDT PRINTDATE 3/1/17

LEGEND

- }} Moderate Seepage
- NC no caving observed
- NS no seepage observed
- 200 Wash (% fines shown)
- Grain Size Analysis (% fines shown)
- Organic Content (% shown)

Excavation Method: Trackhoe
 Date Excavated: December 22, 2016
 Logged By: CJ

Test Pit No.: IT-3



BOTHELL_GEO_2015.GLB TEST_PIT_LOG_1/PAGE INFILTRATION TP LOGS.GPJ BOTHELL_GEO 2010 B&TP.GDT PRINTDATE 3/1/17

LEGEND

}} Moderate Seepage	NC no caving observed	200 Wash (% fines shown)
NS no seepage observed		Grain Size Analysis (% fines shown)
		Organic Content (% shown)

Excavation Method: Trackhoe
 Date Excavated: December 23, 2016
 Logged By: CJ

Test Pit No.: IT-4

DEPTH (feet)	Soil Description	USCS/USGS GRAPHICS	SAMPLE NUMBER	SEEPAGE	CAVING	MOISTURE CONTENT		TESTING
						Plastic Limit	Liquid Limit	
0	Location: North Side of Science Building Approximate ground surface elevation: 122.0 feet 5-inches Crushed Rock over Medium dense, moist, gray with oxidation staining, silty SAND with some gravel and scattered organics (roots) with layers of silty, gravelly SAND (Fill).				NC			
5	Very dense, wet, gray, silty, sandy GRAVEL with interbedded cemented layers of silty gravelly SAND Seepage 7 to 7.5 feet - North Sidewall 4-inch cemented layer - N. half of Pit Infiltration Pond Base Seepage - North Sidewall 1.5" Cemented layer at 8.75 to 9 feet PIT Base		G-1 G-2 G-3 G-4					
10	Very dense, wet, gray, silty, sandy GRAVEL with interbedded cemented layers of silty gravelly SAND (Till)		G-5 G-6					
15	Very dense, moist, gray, silty SAND with some gravel "Excavated from 9 to 14 feet below ground surface after PIT" Test pit terminated at approximately 14 feet		G-7					
20								

BOTHELL_GEO_2015.GLB TEST_PIT_LOG_1/PAGE INFILTRATION TP LOGS.GPJ BOTHELL_GEO 2010 B&TP.GDT PRINTDATE 3/1/17

LEGEND

Moderate Seepage NS no seepage observed	NC no caving observed	200 Wash (% fines shown) Grain Size Analysis (% fines shown) Organic Content (% shown)
--	-----------------------	--

Excavation Method: Trackhoe
 Date Excavated: February 20, 2017
 Logged By: KHM

amec foster wheeler
 11810 North Creek Parkway N
 Bothell, Washington 98011



Test Pit No.: TP-1

DEPTH (feet)	Soil Description	USCS/USGS GRAPHICS	SAMPLE NUMBER	SEEPAGE	CAVING	MOISTURE CONTENT			TESTING
						Plastic Limit	Liquid Limit		
0	Location: Science Building Between IT-4 and OW-1 Approximate ground surface elevation: 122.0 feet								
0 - 5	Loose, moist, gray, silty SAND with some gravel (Fill)			NS	NC				
5 - 5.5	Loose, moist, brown to dark brown, silty SAND with some gravel and intermixed with topsoil / roots (Relic Topsoil Horizon)		G-1						
5.5 - 6.5	Loose, moist, light brown, silty SAND, trace gravel								
6.5 - 8.5	Medium dense, moist, gray, silty SAND, trace to some gravel with scattered organics - (rootlets)		G-2						
8.5 - 10.5	Very dense, moist, gray with oxidation staining, silty gravelly SAND intermixed with some cemented layers of very dense silty sand (Till)		G-3 G-4						
10.5 - 20	Test pit terminated at approximately 10.5 feet								

BOTHELL_GEO_2015.GLB TEST_PIT_LOG_1/PAGE INFILTRATION TP LOGS.GPJ BOTHELL_GEO 2010 B&TP.GDT PRINTDATE 3/1/17

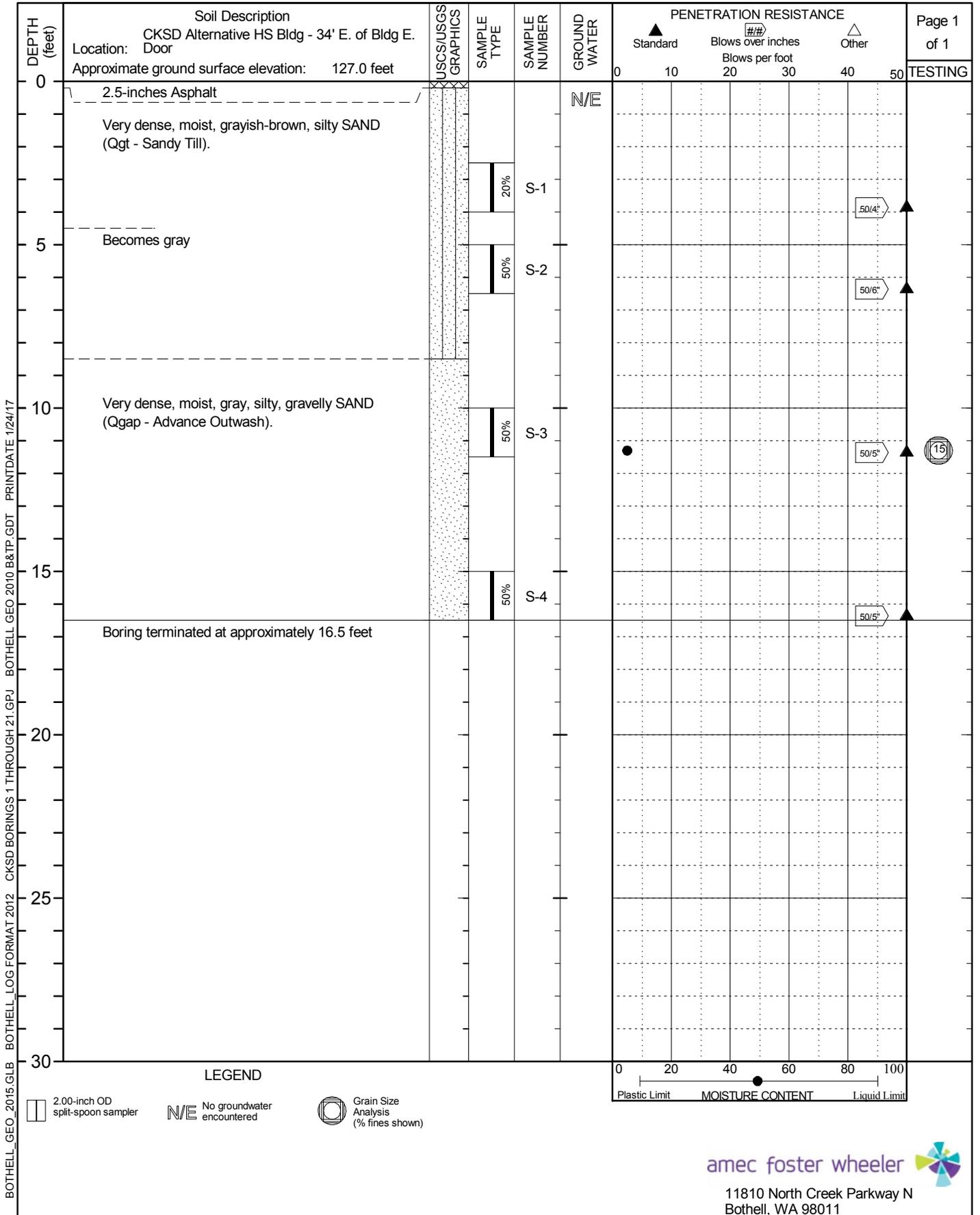
LEGEND

Moderate Seepage	NC no caving observed	200 Wash (% fines shown)
NS no seepage observed		Grain Size Analysis (% fines shown)
		Organic Content (% shown)

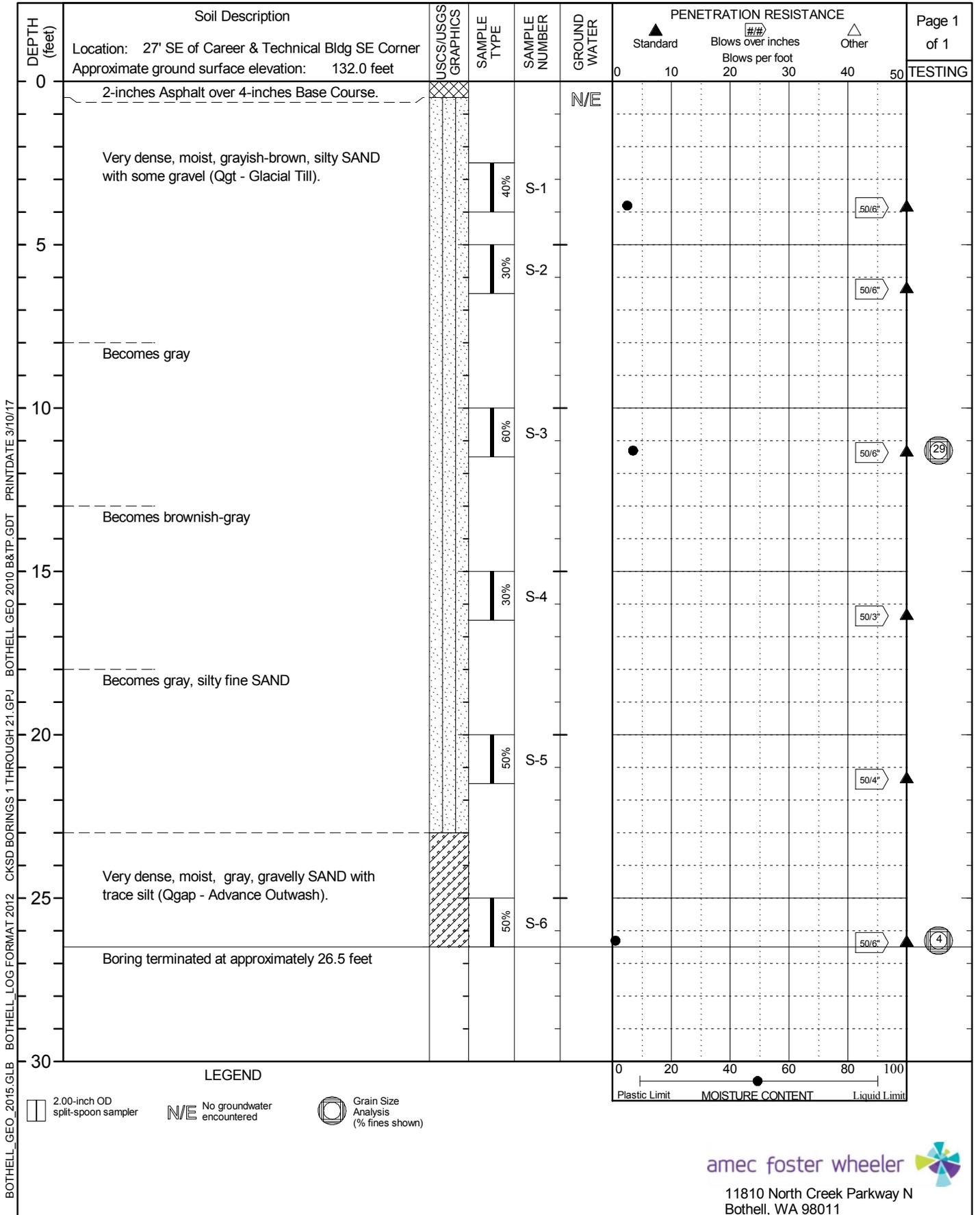
Excavation Method: Trackhoe
 Date Excavated: February 20, 2017
 Logged By: KHM

amec foster wheeler

11810 North Creek Parkway N
 Bothell, Washington 98011

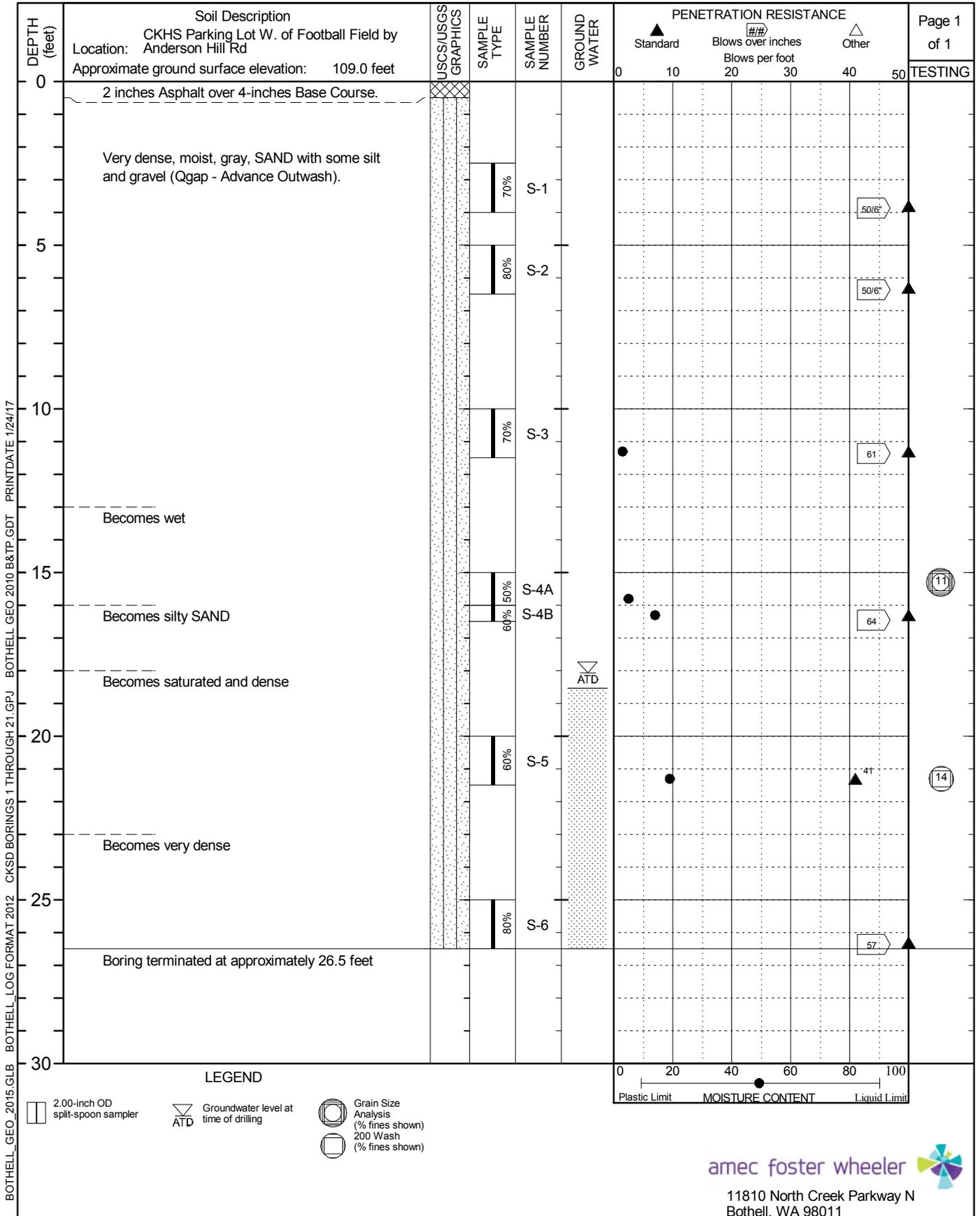


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BOTHELL_GEO_2015.GLB BOTHELL LOG FORMAT 2012 CKSD BORINGS 1 THROUGH 21.GPJ BOTHELL GEO 2010 B&TP.GDT PRINTDATE 3/10/17





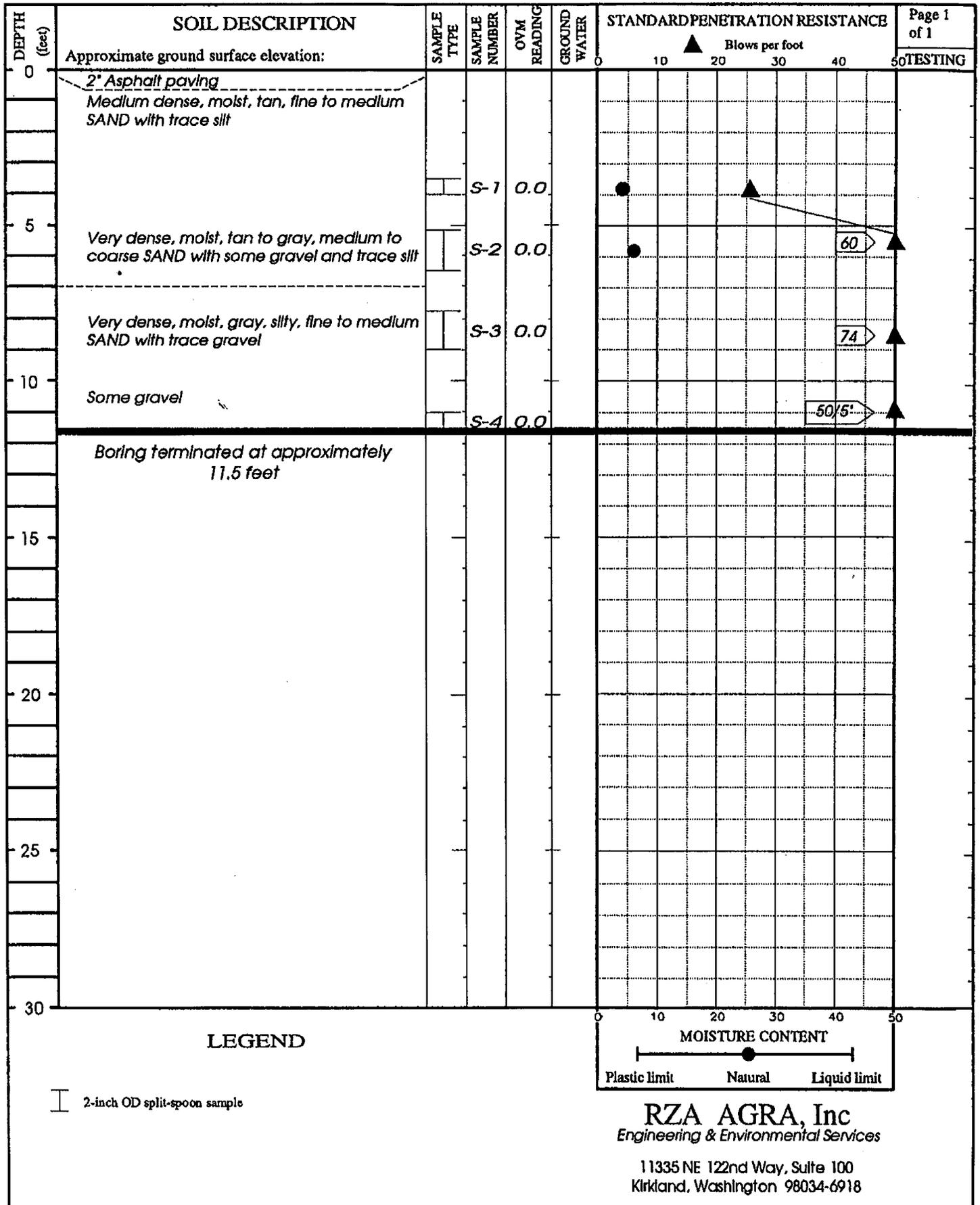
BOTHELL_GEO_2015.GLB BOTHELL_LOG_FORMAT_2012 CKSD BORINGS 1 THROUGH 21.GPJ BOTHELL GEO 2010 B&TP.GDT PRINTDATE 1/24/17



Performing Arts Center

PROJECT: Science Kit Center Site

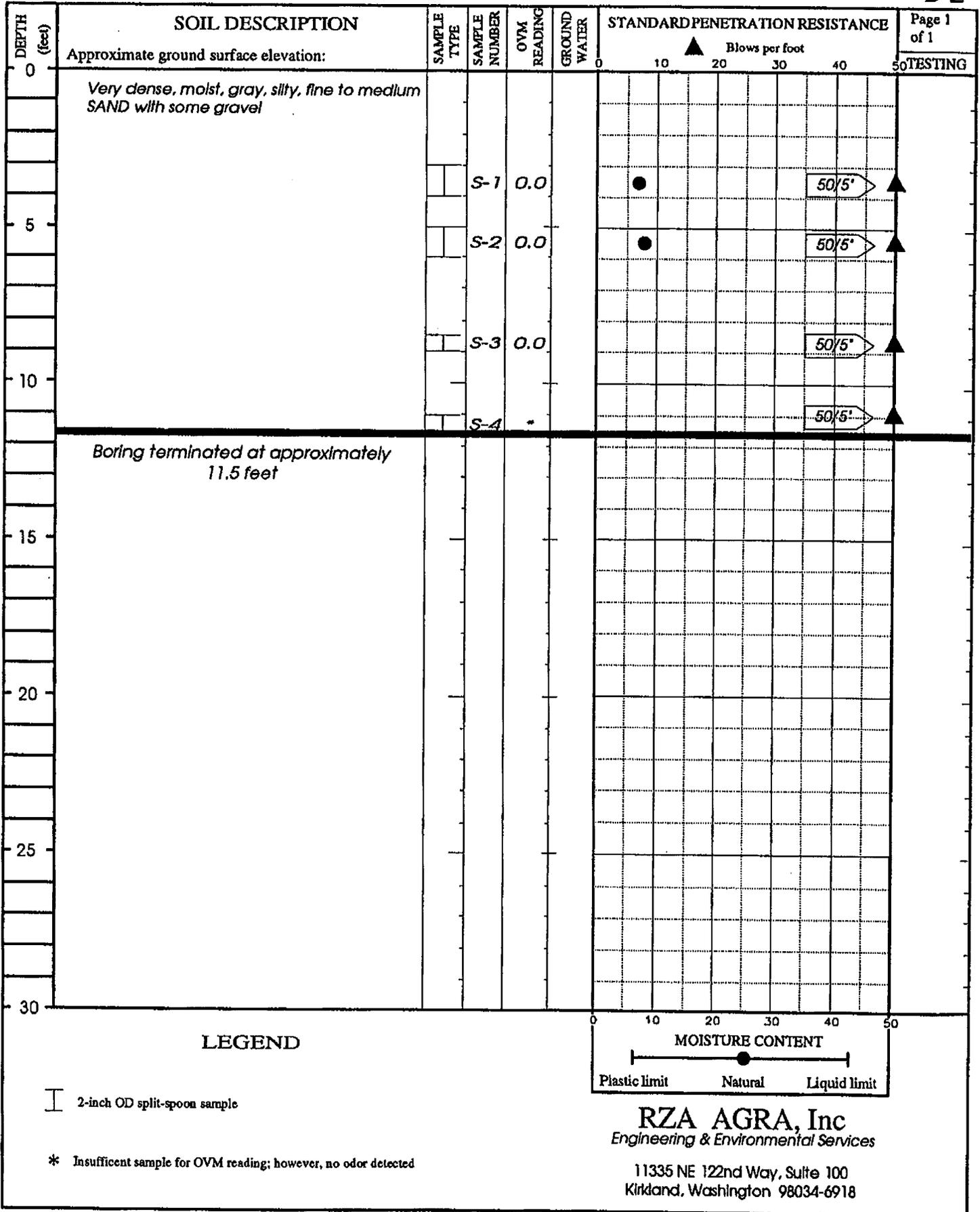
W.O. 11-09290-00 BORING NO. B-1



Performing Arts Center

PROJECT: Science Kit Center Site

W.O. 11-09290-00 BORING NO. B-2



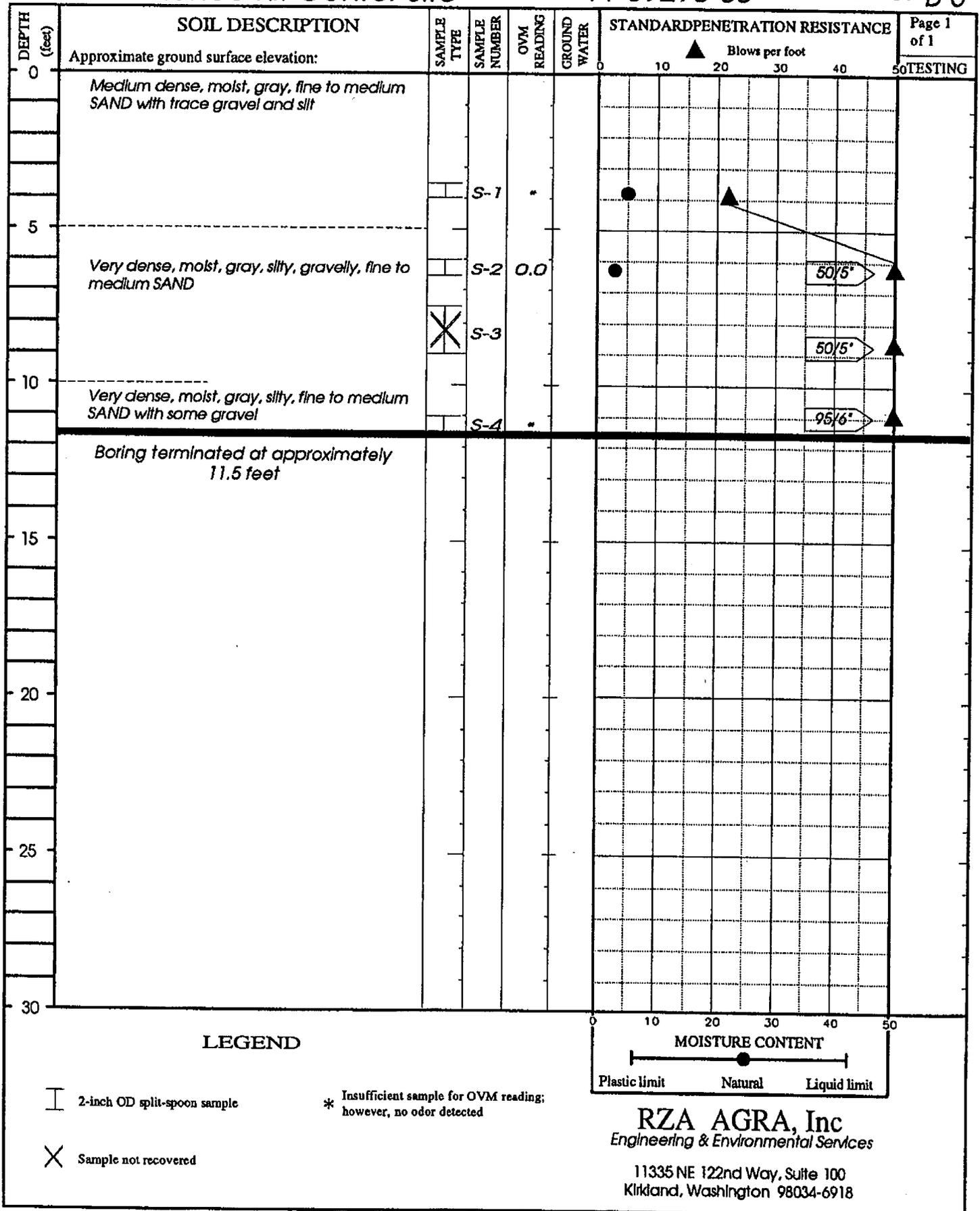
Drilling started: 06 December 1993

Drilling completed: 06 December 1993

Logged by: KHM

Performing Arts Center
PROJECT: Science Kit Center Site

W.O. 11-09290-00 BORING NO. B-3



Drilling started: 06 December 1993

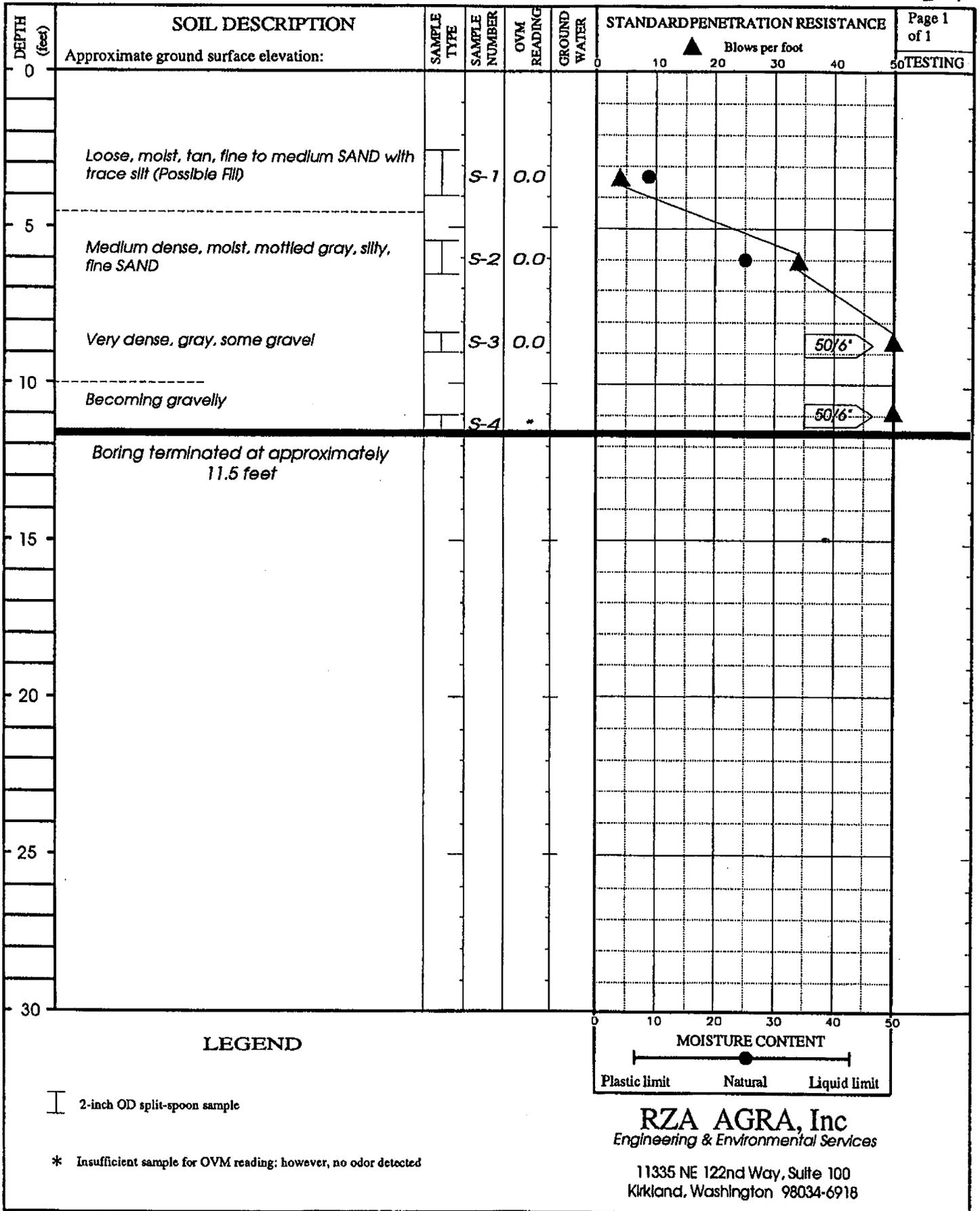
Drilling completed: 06 December 1993

Logged by: KHM

Performing Arts Center

PROJECT: Science Kit Center Site

W.O. 11-09290-00 BORING NO. B-4



Drilling started: 06 December 1993

Drilling completed: 06 December 1993

Logged by: KHM



APPENDIX B

Laboratory Testing Procedures and Results

APPENDIX B

LABORATORY TESTING PROCEDURES AND RESULTS

Central Kitsap High School and Middle School Redevelopment
Silverdale, Washington
Project No. 6-917-18096-0

The following paragraphs describe procedures associated with the laboratory tests conducted for this project. Graphical results of certain laboratory tests are enclosed in this appendix.

VISUAL CLASSIFICATION PROCEDURES

Visual soil classifications were conducted on all samples in the field and on selected samples in the laboratory. All soils were classified in general accordance with the Unified Soil Classification System, which includes color, relative moisture content, primary soil type (based on grain size), and any accessory soil types. The resulting soil classifications are presented on the exploration logs contained in Appendix A.

MOISTURE CONTENT DETERMINATION PROCEDURES

Moisture content determinations were performed on representative samples to aid in identification and correlation of soil types. All determinations were made in general accordance with ASTM D-2216. The results of these tests are shown on the exploration logs in Appendix A.

GRAIN-SIZE ANALYSIS PROCEDURES

A grain-size analysis indicates the range of soil particle diameters included in a particular sample. Grain-size analyses were performed on representative samples in general accordance with ASTM D-422. The results of these tests are presented on the enclosed grain-size distribution graphs and were used in soil classifications shown on the exploration logs in Appendix A.

ORGANIC CONTENT PROCEDURES

Test method ASTM D-2974 covers the measurement of organic matter, ash content, and moisture content in peats and other organic soils, such as organic clays and silts. The results are presented on the enclosed lab data sheet and on the exploration logs presented in Appendix A.

CATION EXCHANGE CAPACITY PROCEDURES

A cation exchange capacity determination was performed on selected samples in general accordance with U.S. Environmental Protection Agency Method 9081. In this procedure, the soil sample is mixed with an excess of sodium acetate solution, resulting in an exchange of the added sodium cations for the matrix cations. The concentration of displaced sodium is then determined by atomic absorption, emission spectroscopy, or an equivalent means. The results are presented in this appendix.

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	1.7	2.1	1.6	18.0	66.3	10.3	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	98.3		
5/8"	98.3		
1/2"	97.7		
3/8"	97.2		
1/4"	96.5		
#4	96.2		
#10	94.6		
#20	91.0		
#40	76.6		
#60	46.6		
#100	21.1		
#200	10.3		

Material Description

Poorly graded sand with silt
As Received Moisture: 8.3%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients
 D₈₅= 0.5511 D₆₀= 0.3110
 D₅₀= 0.2641 D₃₀= 0.1852 D₁₅= 0.1182
 D₁₀= C_u= C_c=

Classification
 USCS= SP-SM AASHTO= A-3

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 12/7/16
 Sampled By: Kori B.

* (no specification provided)

Location: OW-1 / S-3 Depth: 16' Date: 12/19/2016
 Sample Number: 7240.1

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District</p> <p>Project: Central Kitsap HS/MS</p> <p>Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
--	--

Tested By: Jeff W Checked By: Dave D

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	1.5	5.0	4.3	19.6	64.3	5.3	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	98.5		
5/8"	98.5		
1/2"	98.5		
3/8"	96.9		
1/4"	95.2		
#4	93.5		
#10	89.2		
#20	83.6		
#40	69.6		
#60	36.9		
#100	14.9		
#200	5.3		

Material Description

Poorly graded sand with silt
As Received Moisture: 14.6%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients

D ₉₀ = 2.3533	D ₈₅ = 1.0400	D ₆₀ = 0.3577
D ₅₀ = 0.3067	D ₃₀ = 0.2209	D ₁₅ = 0.1504
D ₁₀ = 0.1190	C _u = 3.01	C _c = 1.15

Classification
 USCS= SP-SM AASHTO= A-3

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 12/7/16
 Sampled By: Kori B.

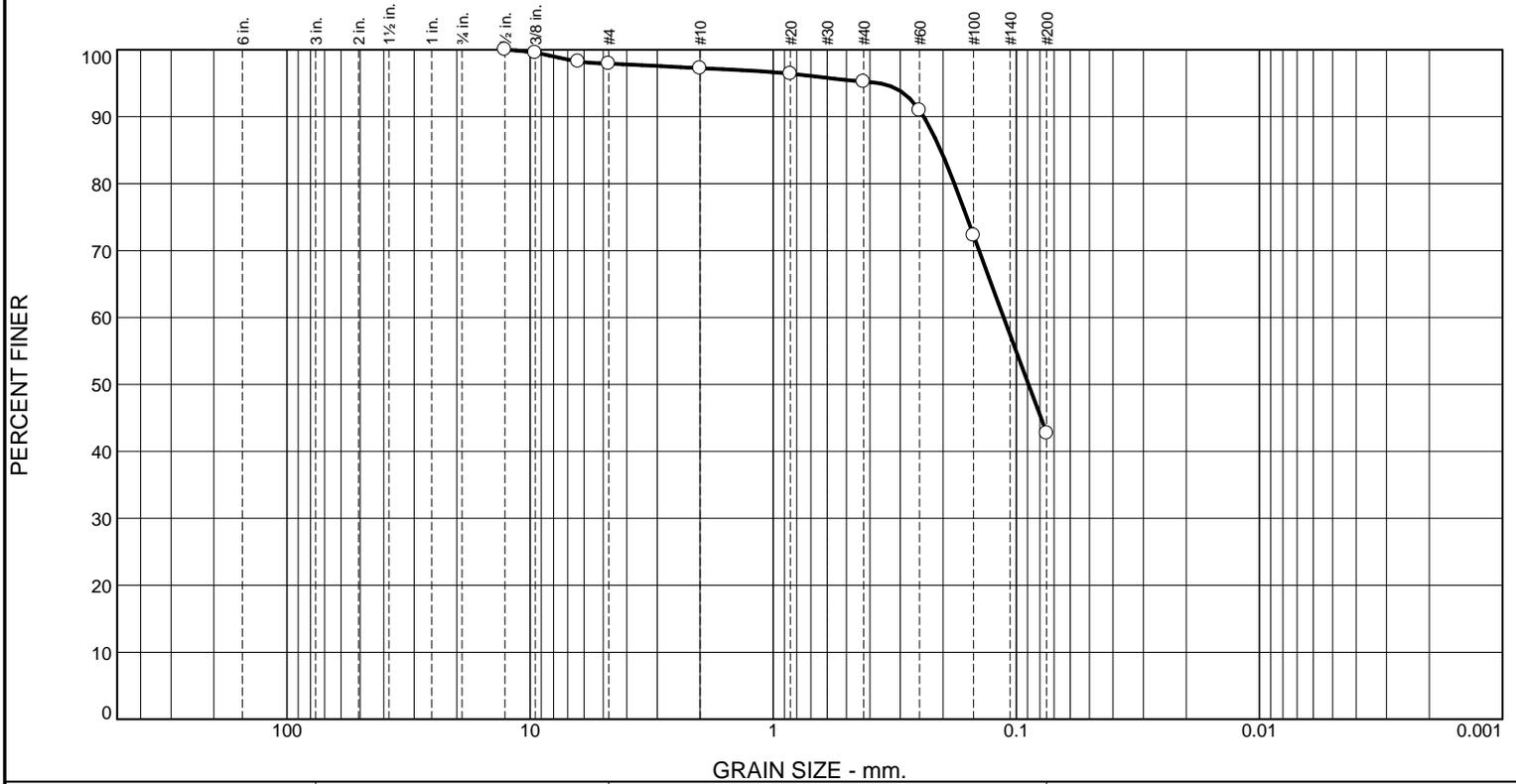
* (no specification provided)

Location: OW-1 - S-6 Depth: 30' Date: 12/19/2016
 Sample Number: 7240.2

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District</p> <p>Project: Central Kitsap HS/MS</p> <p>Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
--	--

Tested By: Jeff W Checked By: Dave D

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	2.1	0.7	2.0	52.5	42.7	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1/2"	100.0		
3/8"	99.5		
1/4"	98.2		
#4	97.9		
#10	97.2		
#20	96.4		
#40	95.2		
#60	91.0		
#100	72.3		
#200	42.7		

Material Description

Silty sand
As Received Moisture: 12.4%

PL= NP	Atterberg Limits	PI= NP
	LL= NV	
	Coefficients	
D ₉₀ = 0.2404	D ₈₅ = 0.2047	D ₆₀ = 0.1127
D ₅₀ = 0.0891	D ₃₀ =	D ₁₅ =
D ₁₀ =	C _u =	C _c =
	Classification	
USCS= SM	AASHTO=	A-4(0)
Remarks		
ASTM: C136, D1140, D2216		
Sampled: 12/7/16		
Sampled By: Kori B.		

* (no specification provided)

Location: OW-2 / S-2B Depth: 11' Date: 12/19/2016
 Sample Number: 7240.3

Terracon Consultants, Inc.	Client: Central Kitsap School District	
Mountlake Terrace, WA	Project: Central Kitsap HS/MS	
	Project No: 6-917-18096-0	Figure

Tested By: Jeff W Checked By: Dave D

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	9.7	6.9	23.7	51.6	8.1	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
5/8"	100.0		
1/2"	98.3		
3/8"	96.8		
1/4"	92.9		
#4	90.3		
#10	83.4		
#20	74.5		
#40	59.7		
#60	34.4		
#100	15.3		
#200	8.1		

Material Description

Poorly graded sand with silt
As Received Moisture: 12.5%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients
 D₉₀= 4.5965 D₈₅= 2.4125 D₆₀= 0.4280
 D₅₀= 0.3414 D₃₀= 0.2276 D₁₅= 0.1479
 D₁₀= 0.1070 C_u= 4.00 C_c= 1.13

Classification
 USCS= SP-SM AASHTO= A-3

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 12/7/16
 Sampled By: Kori B.

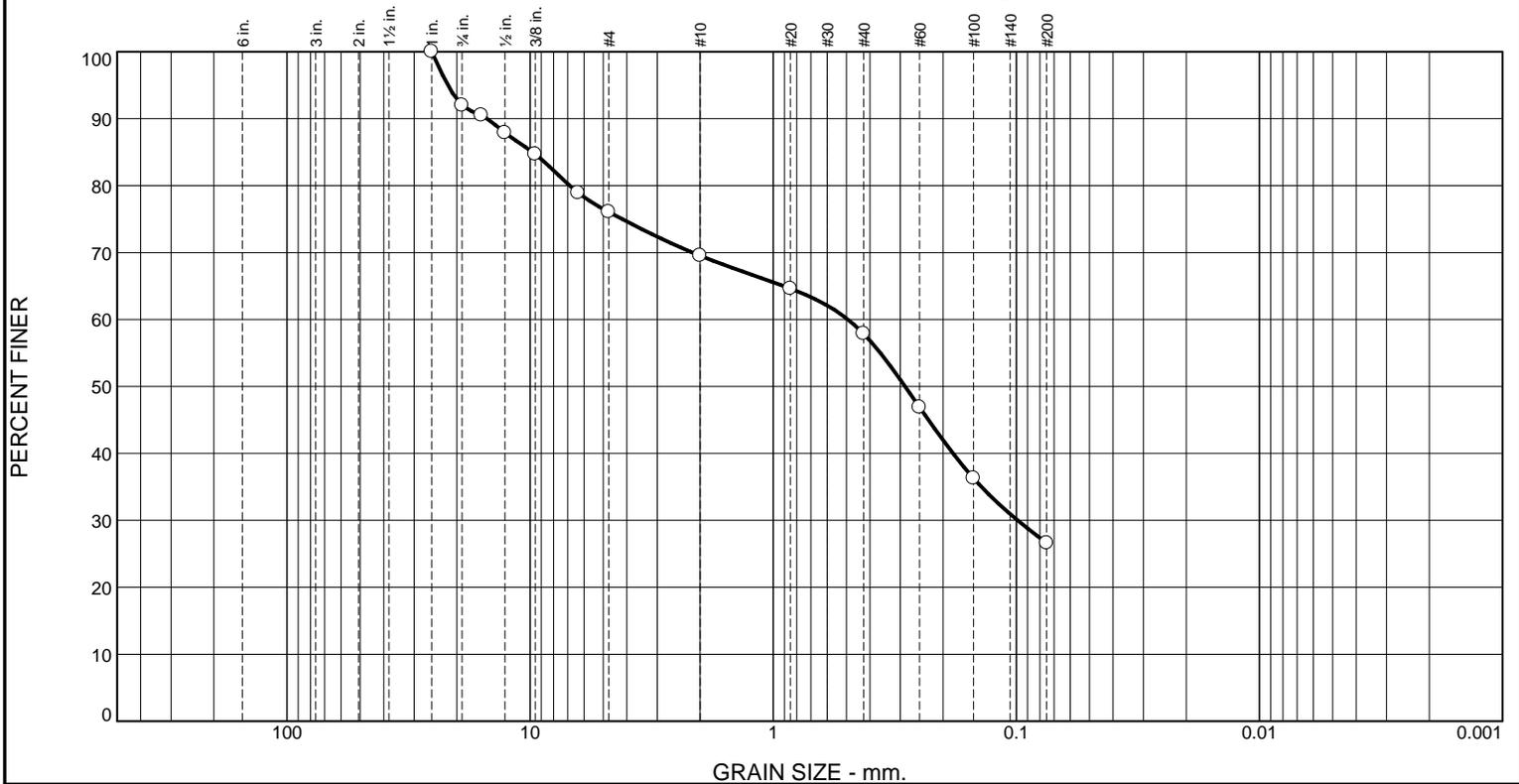
* (no specification provided)

Location: OW-2 / S-3 Sample Number: 7240.4 Depth: 15' Date: 12/19/2016

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District</p> <p>Project: Central Kitsap HS/MS</p> <p>Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
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Tested By: Jeff W Checked By: Dave D

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	8.0	15.9	6.6	11.6	31.3	26.6	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	92.0		
5/8"	90.5		
1/2"	87.9		
3/8"	84.7		
1/4"	78.9		
#4	76.1		
#10	69.5		
#20	64.6		
#40	57.9		
#60	46.9		
#100	36.3		
#200	26.6		

Material Description

Silty sand with gravel
As Received Moisture: 7.4%

Atterberg Limits
 PL= NP LL= NV PI= NP

Coefficients
 D₉₀= 15.0401 D₈₅= 9.7663 D₆₀= 0.4928
 D₅₀= 0.2869 D₃₀= 0.0987 D₁₅=
 D₁₀= C_u= C_c=

Classification
 USCS= SM AASHTO= A-2-4(0)

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 12/7/16
 Sampled By: Kori B.

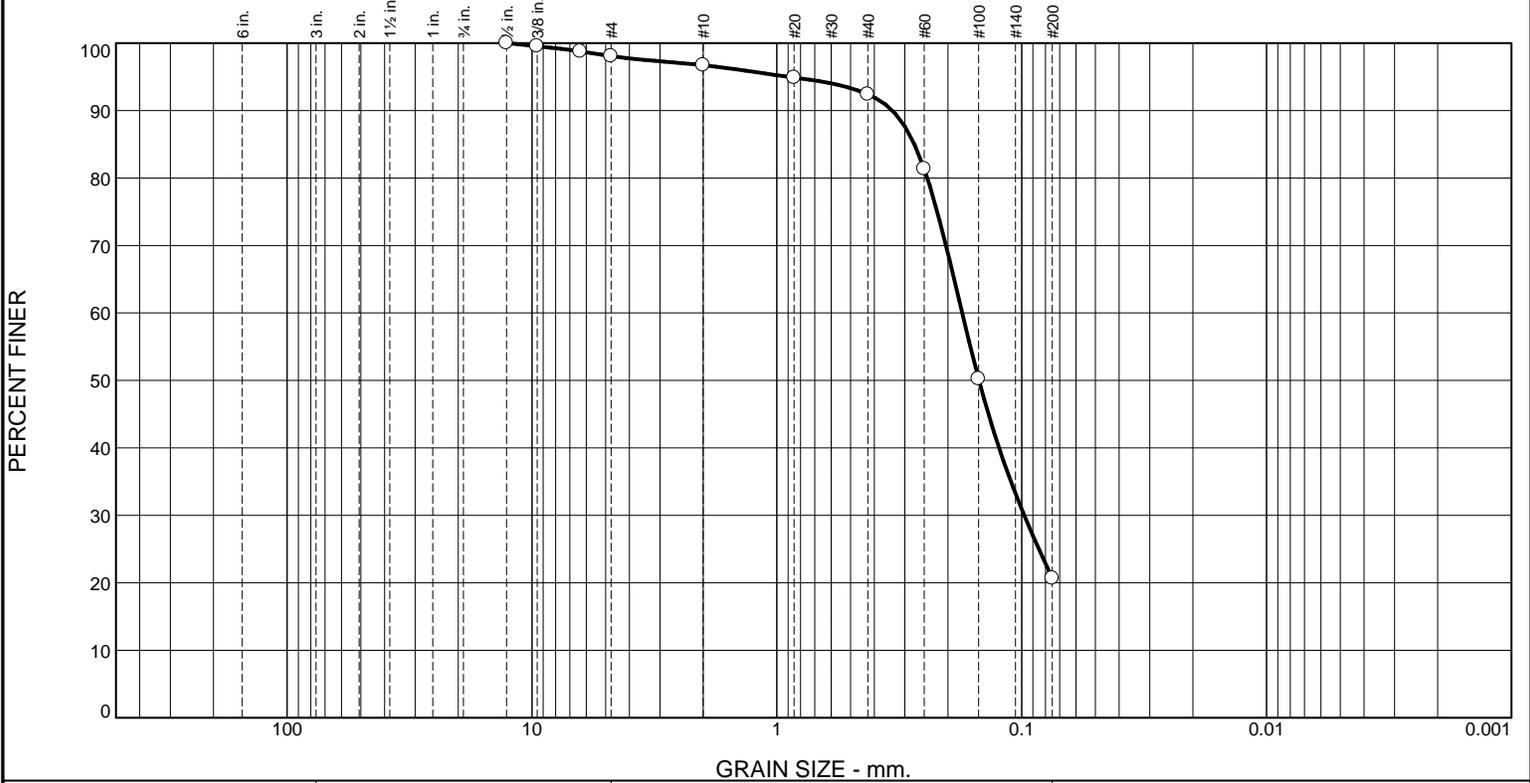
* (no specification provided)

Location: OW-3 / S-2 Sample Number: 7240.5 Depth: 10' Date: 12/19/2016

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District</p> <p>Project: Central Kitsap HS/MS</p> <p>Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
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Tested By: Jeff W Checked By: Dave D

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	1.9	1.4	4.3	71.7	20.7	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1/2"	100.0		
3/8"	99.5		
1/4"	98.8		
#4	98.1		
#10	96.7		
#20	94.9		
#40	92.4		
#60	81.4		
#100	50.2		
#200	20.7		

Material Description

Silty sand
As Received Moisture: 7.5%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients
 D₉₀= 0.3373 D₈₅= 0.2743 D₆₀= 0.1749
 D₅₀= 0.1495 D₃₀= 0.0976 D₁₅=
 D₁₀= C_u= C_c=

Classification
 USCS= SM AASHTO= A-2-4(0)

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 12/7/16
 Sampled By: Kori B.

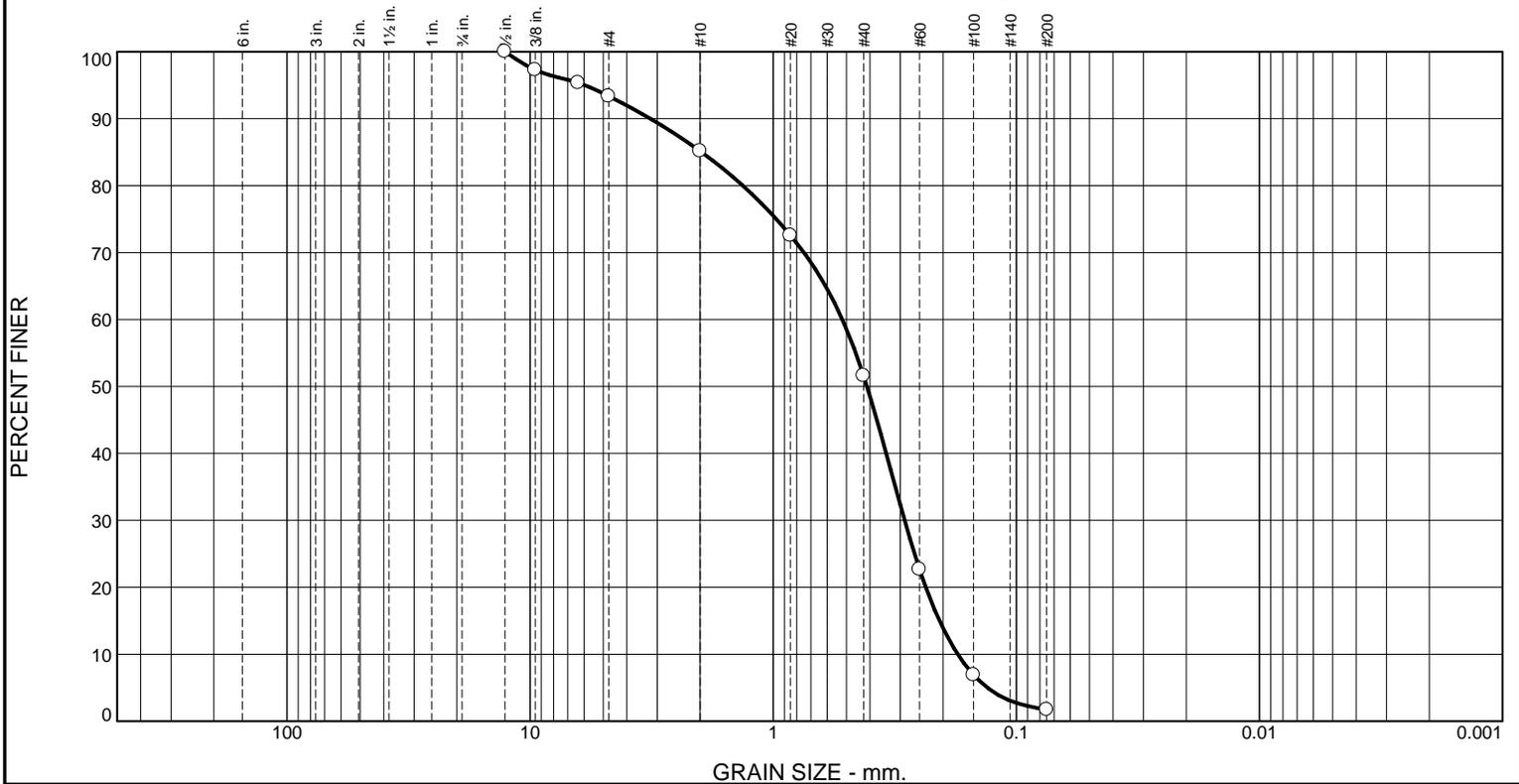
* (no specification provided)

Location: OW-3 / S-4 Depth: 20' Date: 12/19/2016
 Sample Number: 7240.6

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District Project: Central Kitsap HS/MS Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
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Tested By: Jeff W Checked By: Dave D

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	6.6	8.2	33.6	49.9	1.7	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1/2"	100.0		
3/8"	97.3		
1/4"	95.3		
#4	93.4		
#10	85.2		
#20	72.6		
#40	51.6		
#60	22.7		
#100	6.9		
#200	1.7		

Material Description

Poorly graded sand
As Received Moisture: 4.7%

Atterberg Limits
 LL= NV PI= NP

Coefficients
 D₈₅= 1.9723 D₆₀= 0.5197
 D₅₀= 0.4114 D₃₀= 0.2880 D₁₅= 0.2070
 D₁₀= 0.1742 C_u= 2.98 C_c= 0.92

Classification
 USCS= SP AASHTO= A-3

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 12/29/16
 Sampled By: Konrad M.

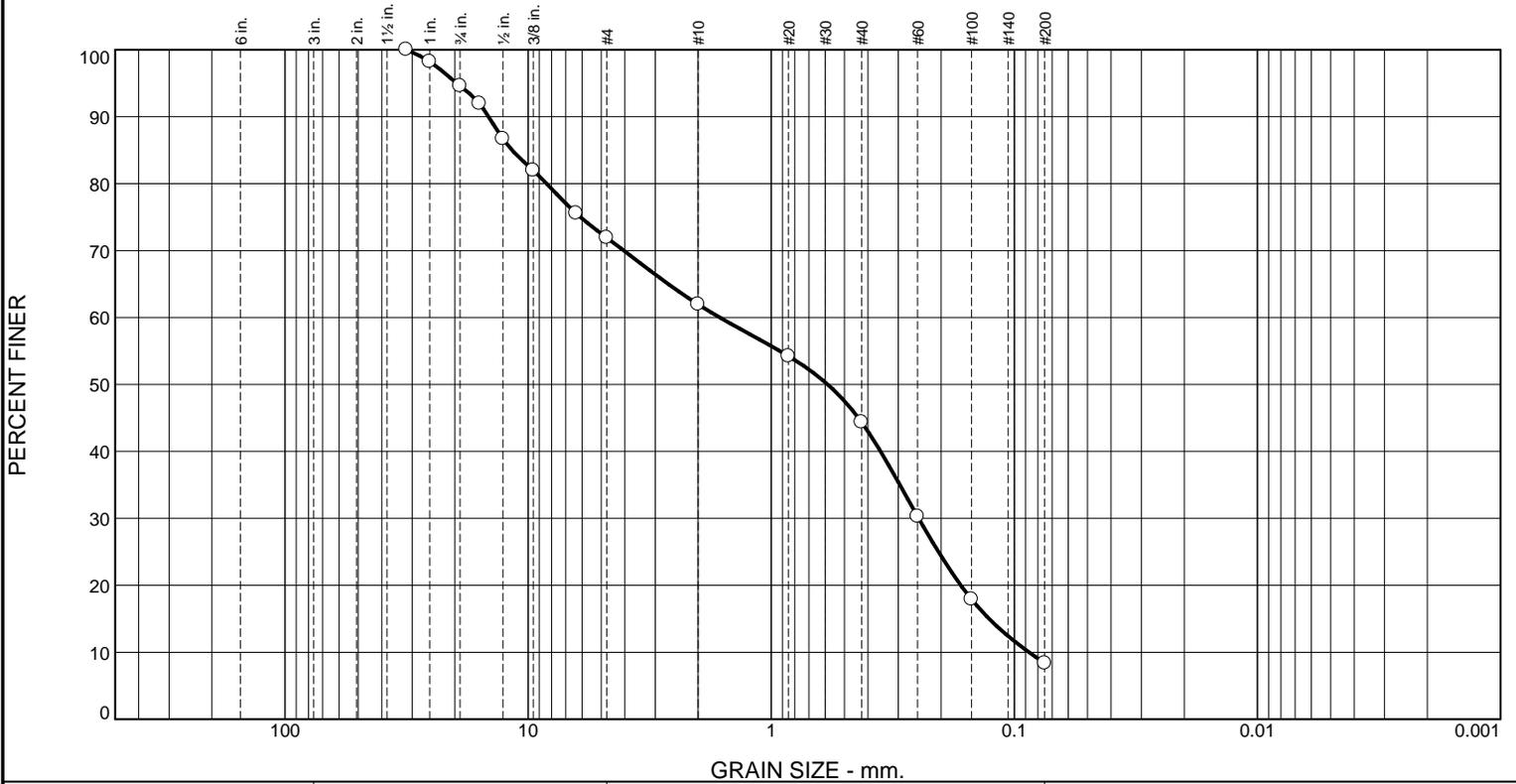
* (no specification provided)

Location: IT-1, G-1 Depth: 7' Date: 1/9/2017
 Sample Number: 7248.1

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District</p> <p>Project: Central Kitsap HS/MS</p> <p>Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
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Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	5.4	22.7	9.9	17.6	36.0	8.4	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.25"	100.0		
1"	98.2		
3/4"	94.6		
5/8"	92.0		
1/2"	86.7		
3/8"	82.0		
1/4"	75.6		
#4	71.9		
#10	62.0		
#20	54.2		
#40	44.4		
#60	30.3		
#100	17.9		
#200	8.4		

Material Description

Poorly graded sand with silt and gravel
As Received Moisture: 5.6%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients

D ₉₀ = 14.5441	D ₈₅ = 11.6485	D ₆₀ = 1.6266
D ₅₀ = 0.5853	D ₃₀ = 0.2474	D ₁₅ = 0.1270
D ₁₀ = 0.0869	C _u = 18.73	C _c = 0.43

Classification
 USCS= SP-SM AASHTO= A-1-b

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 1/3/17
 Sampled By: Konrad M.

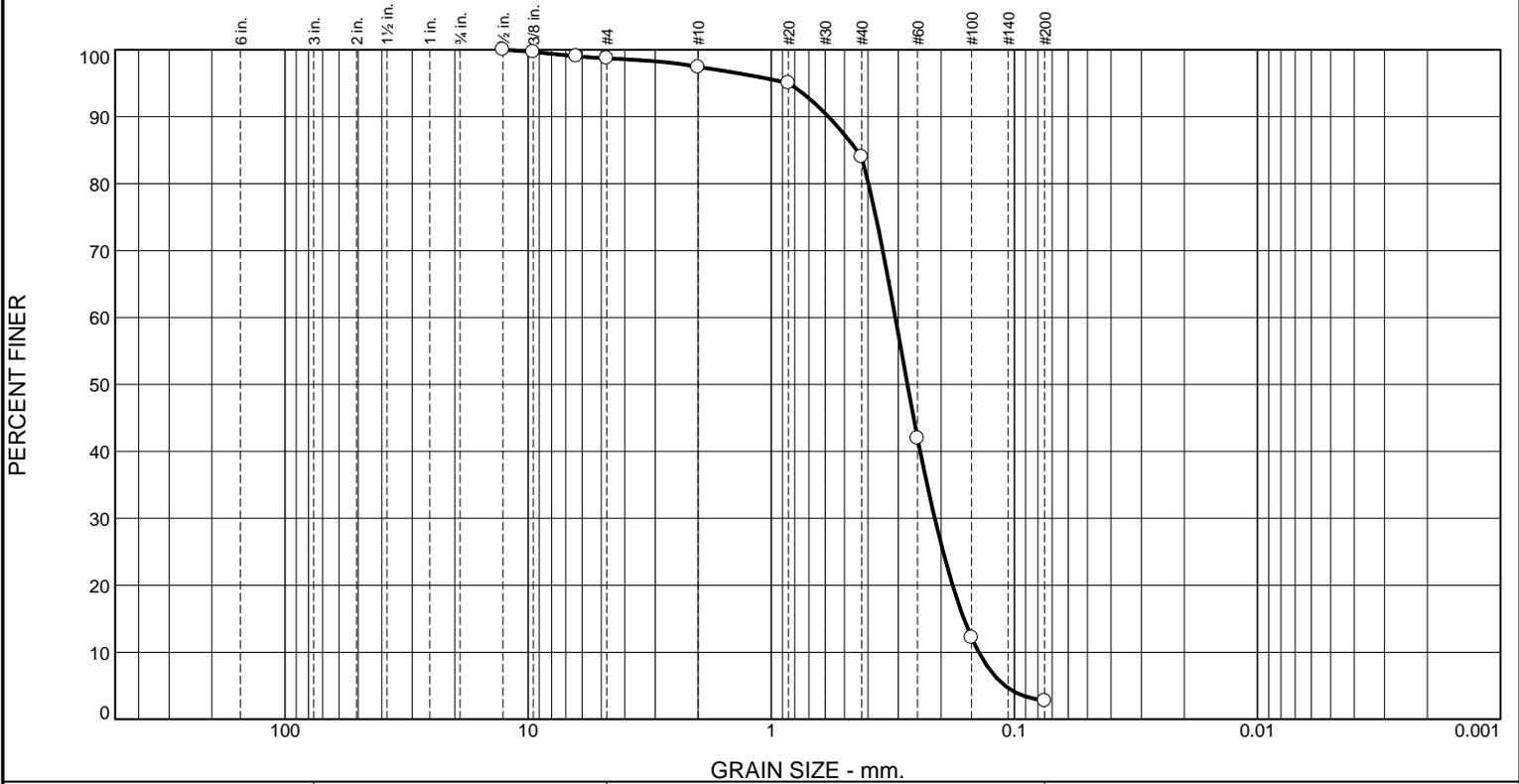
* (no specification provided)

Location: IT-1, G-3 Sample Number: 7250.1 Depth: 11.5' Date: 1/9/2017

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District</p> <p>Project: Central Kitsap HS/MS</p> <p>Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
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Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	1.3	1.3	13.4	81.2	2.8	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1/2"	100.0		
3/8"	99.7		
1/4"	99.0		
#4	98.7		
#10	97.4		
#20	95.0		
#40	84.0		
#60	42.0		
#100	12.2		
#200	2.8		

Material Description

Poorly graded sand
As Received Moisture: 5.1%

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D ₉₀ = 0.5817	D ₈₅ = 0.4453	D ₆₀ = 0.3088
D ₅₀ = 0.2752	D ₃₀ = 0.2124	D ₁₅ = 0.1613
D ₁₀ = 0.1401	C _u = 2.20	C _c = 1.04

Classification

USCS= SP AASHTO= A-3

Remarks

ASTM: C136, D1140, D2216
Sampled: 12/29/16
Sampled By: Konrad M.

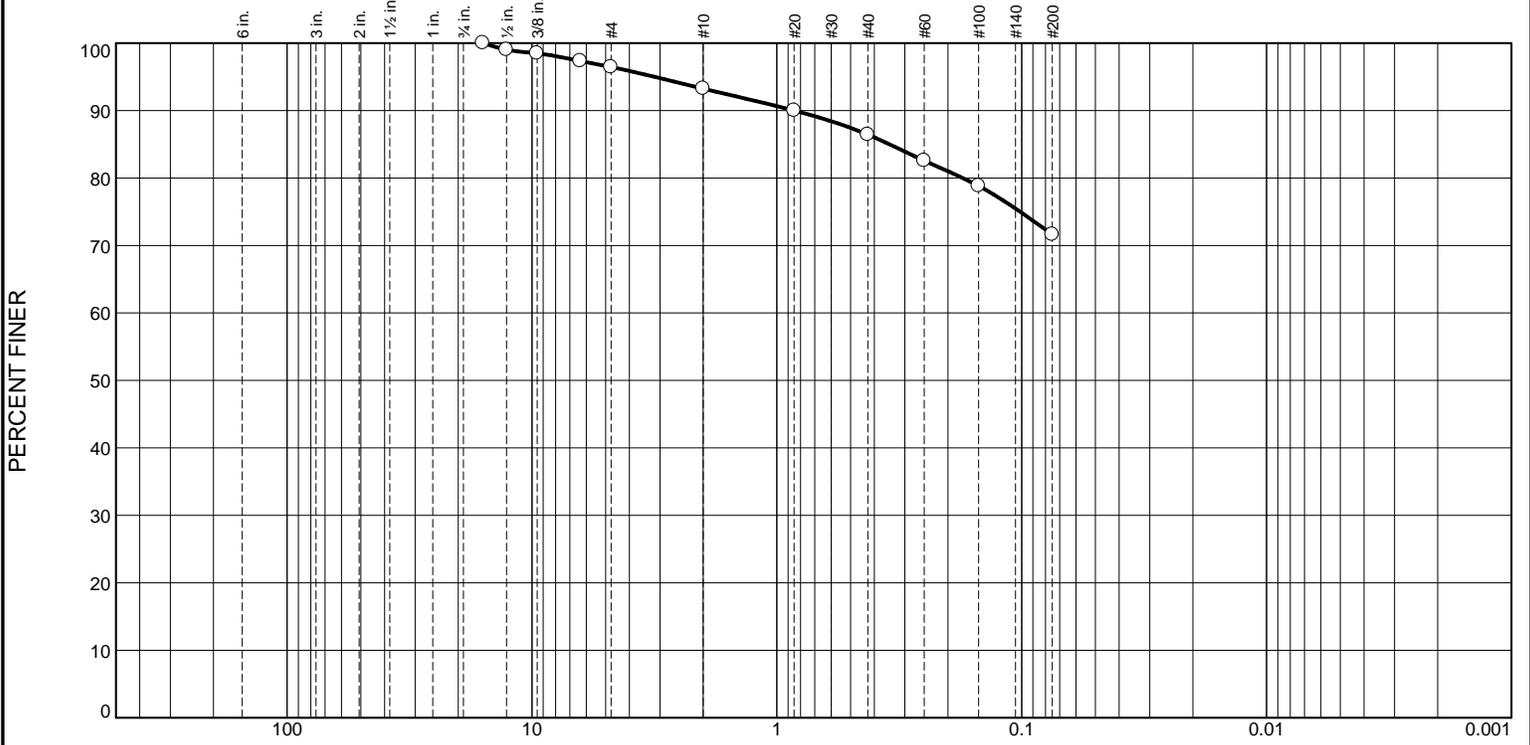
* (no specification provided)

Location: IT-2, G-1 Sample Number: 7248.2 Depth: 9' Date: 1/9/2017

Terracon Consultants, Inc.	Client: Central Kitsap School District	
Mountlake Terrace, WA	Project: Central Kitsap HS/MS	
	Project No: 6-917-18096-0	Figure

Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	3.6	3.2	6.8	14.8	71.6	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
5/8"	100.0		
1/2"	99.0		
3/8"	98.5		
1/4"	97.3		
#4	96.4		
#10	93.2		
#20	90.0		
#40	86.4		
#60	82.6		
#100	78.8		
#200	71.6		

Material Description

Silt with sand
As Received Moisture: 18.1%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients
 D₉₀= 0.8500 D₈₅= 0.3468 D₆₀=
 D₅₀= D₃₀= D₁₅=
 D₁₀= C_u= C_c=

Classification
 USCS= ML AASHTO= A-4(0)

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 1/3/17
 Sampled By: Konrad M.

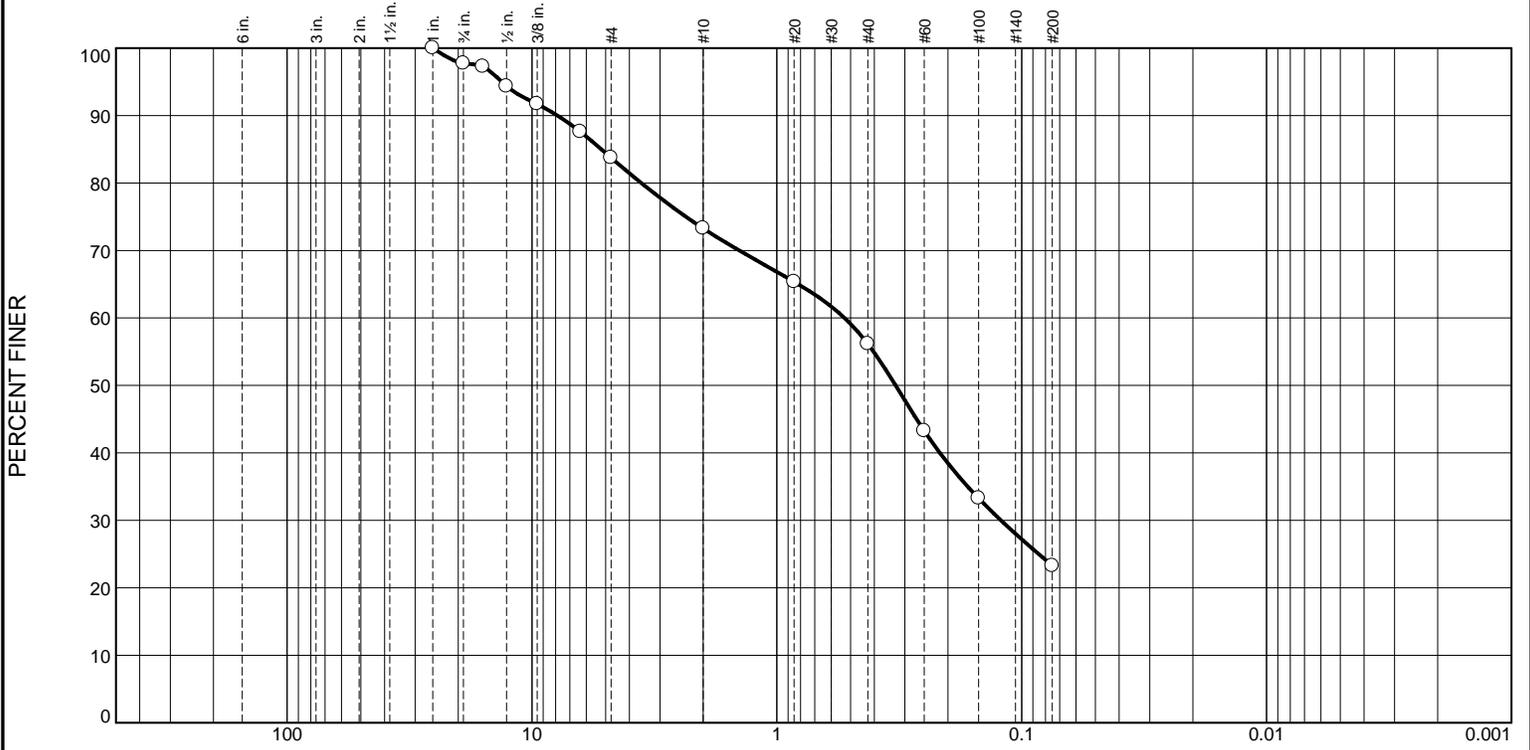
* (no specification provided)

Location: IT-2, G-3 Sample Number: 7250.2 Depth: 12.5' Date: 1/9/2017

Terracon Consultants, Inc.	Client: Central Kitsap School District Project: Central Kitsap HS/MS	
Mountlake Terrace, WA	Project No: 6-917-18096-0	Figure

Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



GRAIN SIZE - mm.

% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	2.3	14.0	10.4	17.1	32.9	23.3	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	97.7		
5/8"	97.3		
1/2"	94.3		
3/8"	91.7		
1/4"	87.6		
#4	83.7		
#10	73.3		
#20	65.4		
#40	56.2		
#60	43.3		
#100	33.3		
#200	23.3		

* (no specification provided)

Material Description

Silty sand with gravel
As Received Moisture: 9.1%

Atterberg Limits	LL= NV	PI= NP
Coefficients	D ₈₅ = 5.2095	D ₆₀ = 0.5290
	D ₅₀ = 0.3272	D ₁₅ =
	D ₁₀ =	C _c =
	C _u =	
Classification	USCS= SM	AASHTO= A-2-4(0)

Remarks

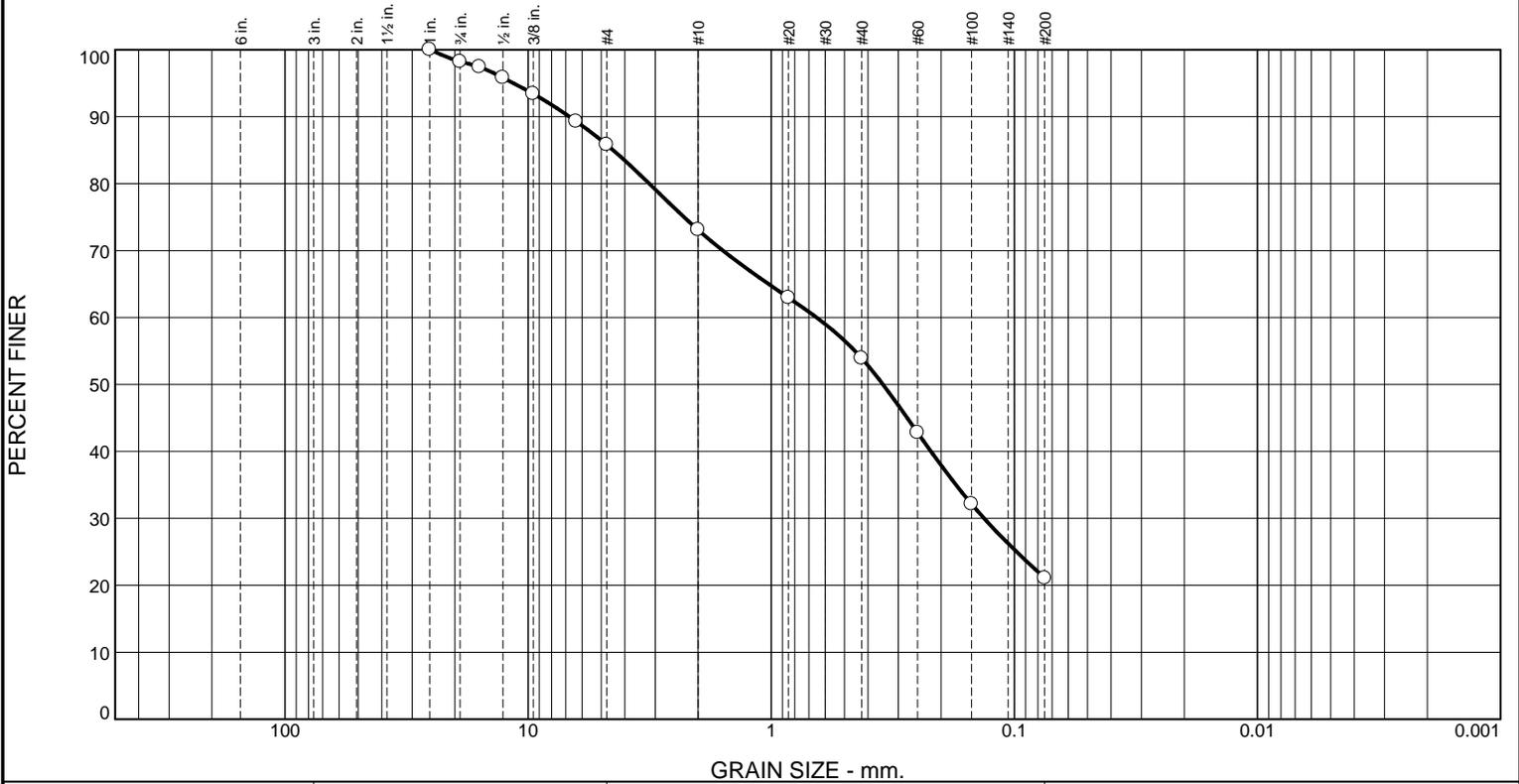
ASTM: C136, D1140, D2216
Sampled: 12/29/16
Sampled By: Konrad M.

Location: IT-3, G-1 Sample Number: 7248.3 Depth: 9' Date: 1/9/2017

Terracon Consultants, Inc.	Client: Central Kitsap School District	
Mountlake Terrace, WA	Project: Central Kitsap HS/MS	
	Project No: 6-917-18096-0	Figure

Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	1.8	12.4	12.7	19.2	32.8	21.1	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	98.2		
5/8"	97.4		
1/2"	95.8		
3/8"	93.4		
1/4"	89.3		
#4	85.8		
#10	73.1		
#20	63.0		
#40	53.9		
#60	42.8		
#100	32.1		
#200	21.1		

* (no specification provided)

Material Description

Silty sand
As Received Moisture: 9.6%

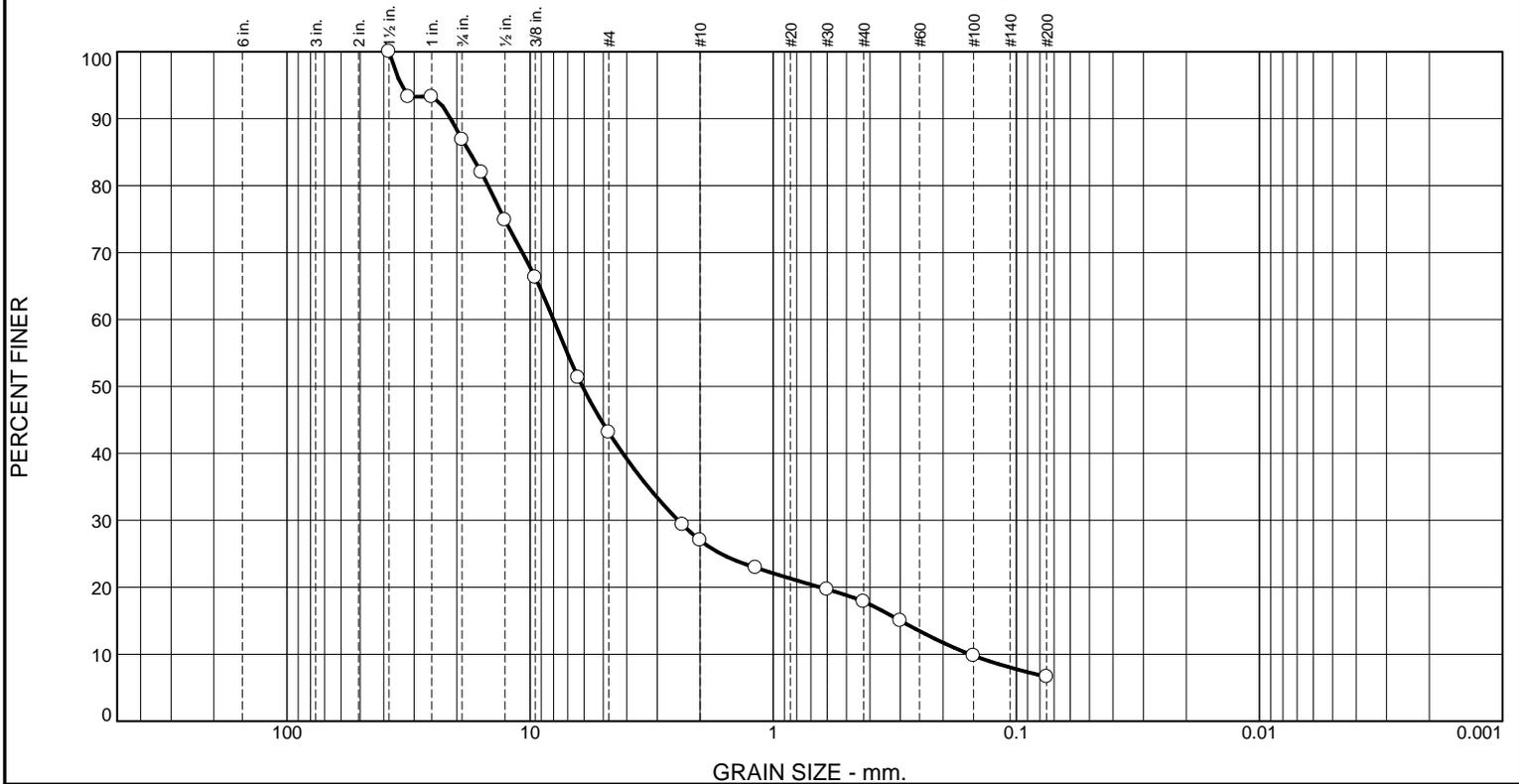
PL= NP	Atterberg Limits	PI=
	LL= NV	
	Coefficients	
D ₉₀ = 6.7671	D ₈₅ = 4.4703	D ₆₀ = 0.6502
D ₅₀ = 0.3470	D ₃₀ = 0.1332	D ₁₅ =
D ₁₀ =	C _u =	C _c =
	Classification	
USCS= SM	AASHTO=	A-2-4(0)
Remarks		
ASTM: C136, D1140, D2216		
Sampled: 1/3/17		
Sampled By: Konrad M.		

Location: IT-3, G-2 Sample Number: 7250.3 Depth: 11.25' Date: 1/9/2017

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District</p> <p>Project: Central Kitsap HS/MS</p> <p>Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
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Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	13.1	43.8	16.1	9.1	11.3	6.6	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.5"	100.0		
1.25"	93.3		
1"	93.3		
3/4"	86.9		
5/8"	82.0		
1/2"	74.8		
3/8"	66.3		
1/4"	51.3		
#4	43.1		
#8	29.4		
#10	27.0		
#16	22.9		
#30	19.7		
#40	17.9		
#50	15.0		
#100	9.8		
#200	6.6		

* (no specification provided)

Material Description

Poorly graded gravel with silt and sand
As Received Moisture: 5.9%

PL= NP	Atterberg Limits	PI= NP
	LL= NV	
	Coefficients	
D ₉₀ = 21.2355	D ₈₅ = 17.7658	D ₆₀ = 8.0122
D ₅₀ = 6.0980	D ₃₀ = 2.4585	D ₁₅ = 0.2994
D ₁₀ = 0.1551	C _u = 51.67	C _c = 4.87
Classification		
USCS= GP-GM	AASHTO=	A-1-a
Remarks		
ASTM: C136, D1140, D2216		
Sampled: 2/21/17		
Sampled by: Konrad M.		

Location: IT-4, G-4
Sample Number: 9919

Depth: 9-10'

Date: 3/1/2017

Terracon Consultants, Inc.

Client: Central Kitsap School District

Project: Central Kitsap HS/MS

Mountlake Terrace, WA

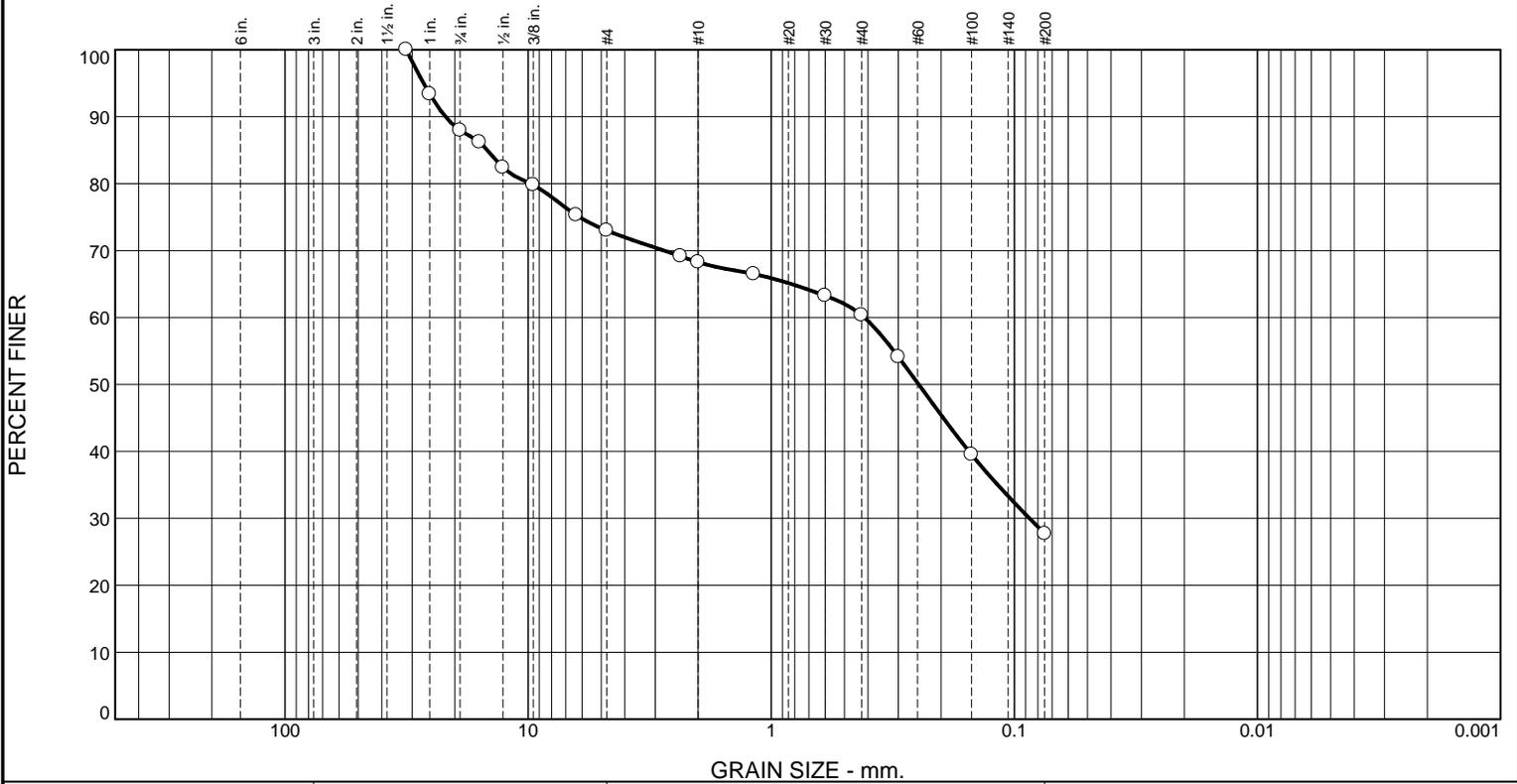
Project No: 6-917-18096-0

Figure

Tested By: Kinsey B

Checked By: Jeff W

Particle Size Distribution Report



GRAIN SIZE - mm.

% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	12.1	14.9	4.7	7.9	32.7	27.7	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.25"	100.0		
1"	93.4		
3/4"	87.9		
5/8"	86.2		
1/2"	82.4		
3/8"	79.8		
1/4"	75.3		
#4	73.0		
#8	69.2		
#10	68.3		
#16	66.5		
#30	63.3		
#40	60.4		
#50	54.1		
#100	39.5		
#200	27.7		

* (no specification provided)

Material Description

Silty sand with gravel
As Received Moisture: 10.5%

PL= NP	Atterberg Limits LL= NV	PI= NP
Coefficients		
D ₉₀ = 21.8880	D ₈₅ = 14.7151	D ₆₀ = 0.4137
D ₅₀ = 0.2472	D ₃₀ = 0.0866	D ₁₅ =
D ₁₀ =	C _u =	C _c =
Classification		
USCS= SM	AASHTO= A-2-4(0)	
Remarks		
ASTM: C136, D1140, D2216		
Sampled: 2/21/17		
Sampled by: Konrad M.		

Location: IT-4, G-7

Sample Number: 9920

Depth: 13-14'

Date: 3/1/2017

Terracon Consultants, Inc.

Client: Central Kitsap School District

Project: Central Kitsap HS/MS

Mountlake Terrace, WA

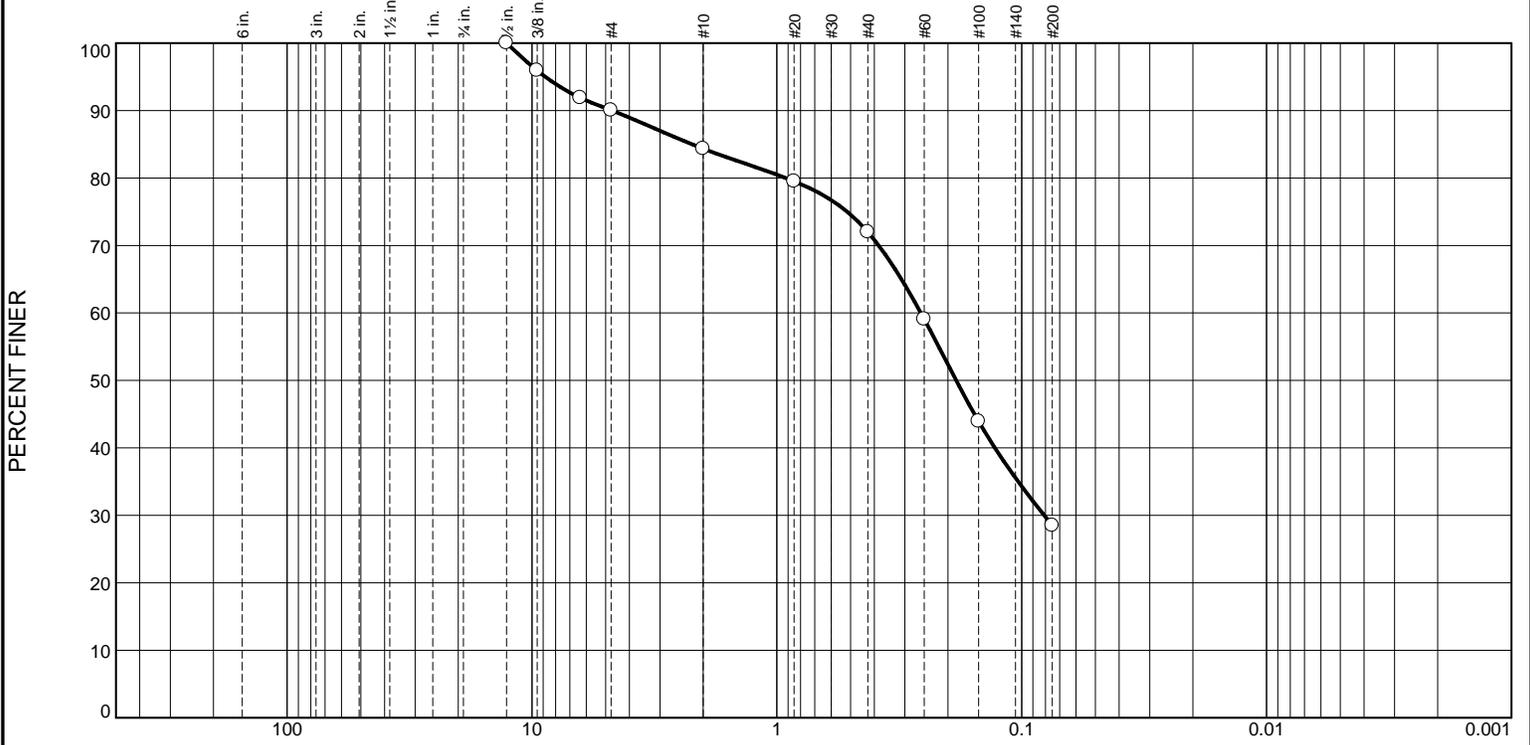
Project No: 6-917-18096-0

Figure

Tested By: Kinsey B

Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	10.0	5.7	12.3	43.5	28.5	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1/2"	100.0		
3/8"	95.9		
1/4"	91.9		
#4	90.0		
#10	84.3		
#20	79.5		
#40	72.0		
#60	59.1		
#100	43.9		
#200	28.5		

Soil Description

Silty sand
As Received Moisture: 6.6%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients
 D₉₀= 4.7143 D₈₅= 2.2209 D₆₀= 0.2581
 D₅₀= 0.1851 D₃₀= 0.0809 D₁₅=
 D₁₀= C_u= C_c=

Classification
 USCS= SM AASHTO= A-2-4(0)

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 8/31/16
 Sampled By: Konrad M. & Frank C.

* (no specification provided)

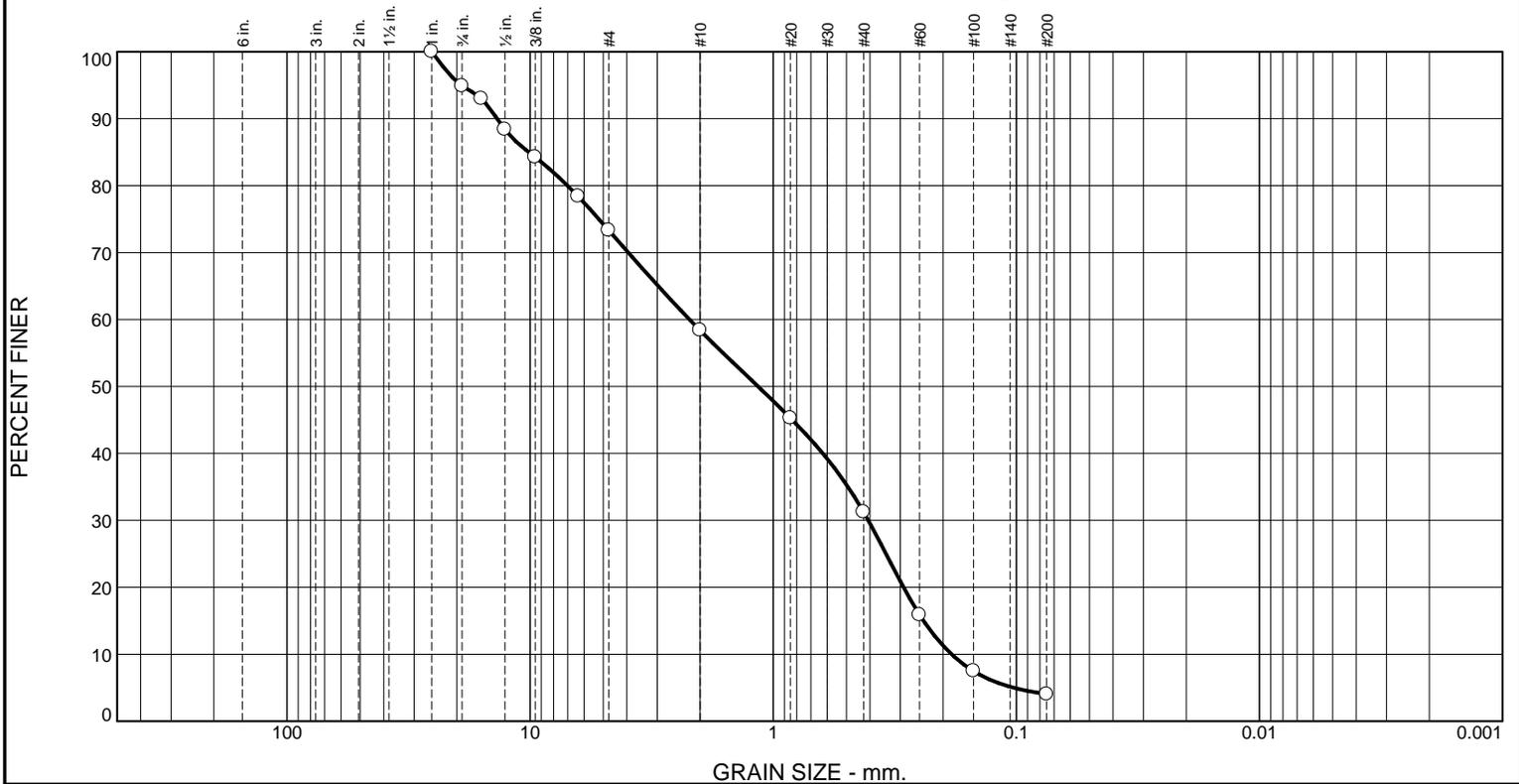
Location: B-11, S-3
 Depth: 10-11.5

Date: 9/14/2016

<p>Terracon Consultants, Inc.</p> <p>Mountlake Terrace, WA</p>	<p>Client: Central Kitsap School District Project: Central Kitsap HS/MS Project No: 6-917-18096-0</p> <p style="text-align: right;">Figure</p>
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Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	5.1	21.6	14.9	27.2	27.2	4.0	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	94.9		
5/8"	93.0		
1/2"	88.4		
3/8"	84.3		
1/4"	78.4		
#4	73.3		
#10	58.4		
#20	45.3		
#40	31.2		
#60	15.9		
#100	7.5		
#200	4.0		

* (no specification provided)

Soil Description

Poorly graded sand with gravel
As Received Moisture: 0.6%

Atterberg Limits

PL= NP LL= NV PI=

Coefficients

D₉₀= 13.6798 D₈₅= 10.1238 D₆₀= 2.2101
D₅₀= 1.1544 D₃₀= 0.4064 D₁₅= 0.2409
D₁₀= 0.1846 C_u= 11.97 C_c= 0.40

Classification

USCS= SP AASHTO= A-1-b

Remarks

ASTM: C136, D1140, D2216
Sampled: 8/31/16
Sampled By: Konrad M. & Frank C.

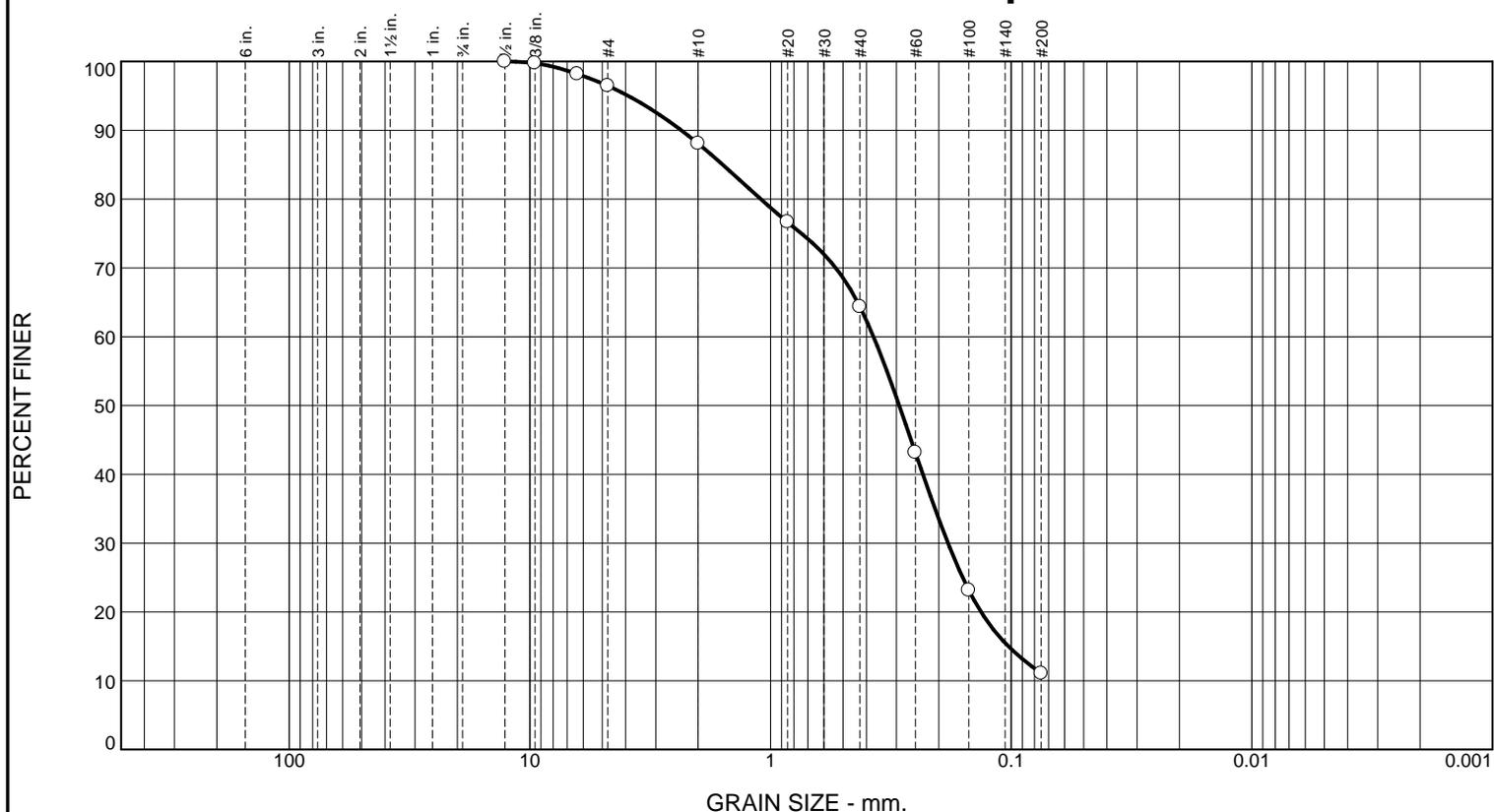
Location: B-11, S-6
Depth: 25-26.5

Date: 9/14/2016

Terracon Consultants, Inc.	Client: Central Kitsap School District	
Mountlake Terrace, WA	Project: Central Kitsap HS/MS	
Project No: 6-917-18096-0	Figure	

Tested By: Ryan G Checked By: Jeff W

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	3.5	8.4	23.7	53.3	11.1	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1/2"	100.0		
3/8"	99.8		
1/4"	98.2		
#4	96.5		
#10	88.1		
#20	76.6		
#40	64.4		
#60	43.2		
#100	23.1		
#200	11.1		

Soil Description

Poorly graded sand with silt
As Received Moisture: 5.2%

Atterberg Limits
 PL= NP LL= NV PI=

Coefficients
 D₉₀= 2.3471 D₈₅= 1.5800 D₆₀= 0.3733
 D₅₀= 0.2919 D₃₀= 0.1830 D₁₅= 0.1027
 D₁₀= C_u= C_c=

Classification
 USCS= SP-SM AASHTO= A-2-4(0)

Remarks
 ASTM: C136, D1140, D2216
 Sampled: 8/31/16
 Sampled By: Konrad M. & Frank C.

* (no specification provided)

Location: B-15, S-4A
Depth: 15-16

Date: 9/19/2016

Terracon Consultants, Inc.

Client: Central Kitsap School District
Project: Central Kitsap HS/MS

Mountlake Terrace, WA

Project No: 6-917-18096-0

Figure

**MOISTURE CONTENT AND MINUS 200 WASH
ASTM: D2216 D1140**

Job Name: Central Kitsap High School/Middle School	Client: Central Kitsap School District
Job Number: 6-917-18096-0	Sample Date: 1/3/2017
Date: 1/9/2017	Sampled By: Konrad M.

Exploration:	IT-1
Sample Number:	G-2
Depth:	9.5'
% Moisture	9.6%
% -200 Wash	17.96%

Tested By: Jeff W.
Reveiwed By: Dave D.
Respectfully submitted,

By: Jeff Ward





Am Test Inc.
13600 NE 126TH PL
Suite C
Kirkland, WA 98034
(425) 885-1664

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Jan 13 2017
AMEC FOSTER WHEELER
11810 NORTH CREEK PKWY N
BOTHELL, WA 98011
Attention: KONRAD MOELLER/TODD WENTWORTH

Dear KONRAD MOELLER/TODD WENTWORTH:

Enclosed please find the analytical data for your CENTRAL KITSAP HS/MS project.

The following is a cross correlation of client and laboratory identifications for your convenience.

CLIENT ID	MATRIX	AMTEST ID	TEST
IT-1	Soil	17-A000151	CONV
IT-2	Soil	17-A000152	CONV
IT-3	Soil	17-A000153	CONV

Your samples were received on Friday, January 6, 2017. At the time of receipt, the samples were logged in and properly maintained prior to the subsequent analysis.

The analytical procedures used at AmTest are well documented and are typically derived from the protocols of the EPA, USDA, FDA or the Army Corps of Engineers.

Following the analytical data you will find the Quality Control (QC) results.

Please note that the detection limits that are listed in the body of the report refer to the Practical Quantitation Limits (PQL's), as opposed to the Method Detection Limits (MDL's).

If you should have any questions pertaining to the data package, please feel free to contact me.

Sincerely,


Aaron W. Young
Laboratory Manager

BACT = Bacteriological
CONV = Conventionals

MET = Metals
ORG = Organics

NUT=Nutrients
DEM=Demand

MIN=Minerals

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ANALYSIS REPORT

AMEC FOSTER WHEELER
11810 NORTH CREEK PKWY N
BOTHELL, WA 98011
Attention: KONRAD MOELLER/TODD WENTWORTH
Project Name: CENTRAL KITSAP HS/MS
All results reported on an as received basis.

Date Received: 01/06/17
Date Reported: 1/13/17

AMTEST Identification Number 17-A000151
Client Identification IT-1
Sampling Date

Conventionals

PARAMETER	RESULT	UNITS	Q	D.L.	METHOD	ANALYST	DATE
Cation Exchange Capacity	2.6	meq/100g		0.5	SW-846 9081	AY	01/12/17
Organic Matter	0.7	%		0.1	ASTM D 2974	SW	01/10/17

AMTEST Identification Number 17-A000152
Client Identification IT-2
Sampling Date

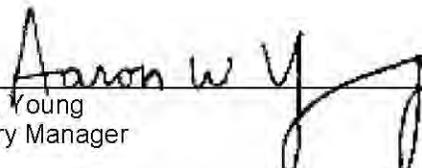
Conventionals

PARAMETER	RESULT	UNITS	Q	D.L.	METHOD	ANALYST	DATE
Cation Exchange Capacity	1.9	meq/100g		0.5	SW-846 9081	AY	01/12/17
Organic Matter	0.5	%		0.1	ASTM D 2974	SW	01/10/17

AMTEST Identification Number 17-A000153
Client Identification IT-3
Sampling Date

Conventionals

PARAMETER	RESULT	UNITS	Q	D.L.	METHOD	ANALYST	DATE
Cation Exchange Capacity	6.7	meq/100g		0.5	SW-846 9081	AY	01/12/17
Organic Matter	1.1	%		0.1	ASTM D 2974	SW	01/10/17



Aaron W. Young
Laboratory Manager

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Kirkland, WA, 98034
(425) 885-1664
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QC Summary for sample numbers: 17-A000151 to 17-A000153

DUPLICATES

SAMPLE #	ANALYTE	UNITS	SAMPLE VALUE	DUP VALUE	RPD
17-A000153	Organic Matter	%	1.1	1.5	31.

STANDARD REFERENCE MATERIALS

ANALYTE	UNITS	TRUE VALUE	MEASURED VALUE	RECOVERY
Cation Exchange Capacity	meq/100g	4.0	4.0	100. %

BLANKS

ANALYTE	UNITS	RESULT
Cation Exchange Capacity	meq/100g	< 0.5



AmTest Chain of Custody Record

13600 NE 126th PL, Suite C, Kirkland, WA 98034

Ph (425) 885-1664 Fx (425) 820-0245

www.amtestlab.com

Chain of Custody No. **30053**

Client Name & Address: <i>Amel Foster Wheeler</i>	Invoice To:
Contact Person: <i>Konrad Moeller Todd Wentworth</i>	Invoice Contact:
Phone No: <i>425-368-1000</i>	PO Number:
Fax No:	Invoice Ph/Fax:
E-mail: <i>todd.wentworth@amctw.com</i> <i>Konrad.moeller@amctw.com</i>	Invoice E-mail:
Report Delivery: (Choose all that apply) Mail / Fax / <u>Email</u> / Posted Online	Data posted to online account: YES / NO Web Login ID:

Special Instructions:

Requested TAT: (Rush must be pre-approved by lab)
 Standard RUSH (5 Day / 3 Day / 48 HR / 24 HR) **Temperature upon Receipt: 13.7**

Project Name: <i>Central Kitsap HS/MS</i>		Date Sampled	Time Sampled	Matrix	No. of containers	Analysis Requested										QA/QC		
Project Number:	AmTest ID					Client ID (35 characters max)	CEC	Organic Content										
	<i>S1</i>	<i>IT-1</i>				+	+											
	<i>S2</i>	<i>IT-2</i>				+	+											
	<i>S3</i>	<i>IT-3</i>				+	+											

Collected/Relinquished By: <i>Ned Moeller</i>	Date <i>1/6/17</i>	Time <i>1140</i>	Received By: <i>RM</i>	Date <i>1/6/17</i>	Time <i>1140</i>
Relinquished By:	Date	Time	Received By:	Date	Time
Relinquished By:	Date	Time	Received By:	Date	Time

COMMENTS: *Client*



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Kirkland, WA 98034
(425) 885-1664

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Mar 1 2017
AMEC FOSTER WHEELER
11810 NORTH CREEK PKWY N
BOTHELL, WA 98011
Attention: KONRAD MUELLER/TODD WENTWORTH

Dear KONRAD MUELLER/TODD WENTWORTH:

Enclosed please find the analytical data for your CENTRAL KITSAP HS/MS project.

The following is a cross correlation of client and laboratory identifications for your convenience.

CLIENT ID	MATRIX	AMTEST ID	TEST
IT-4/G-4E-9	Soil	17-A002444	CONV

Your sample was received on Wednesday, February 22, 2017. At the time of receipt, the sample was logged in and properly maintained prior to the subsequent analysis.

The analytical procedures used at AmTest are well documented and are typically derived from the protocols of the EPA, USDA, FDA or the Army Corps of Engineers.

Following the analytical data you will find the Quality Control (QC) results.

Please note that the detection limits that are listed in the body of the report refer to the Practical Quantitation Limits (PQL's), as opposed to the Method Detection Limits (MDL's).

If you should have any questions pertaining to the data package, please feel free to contact me.

Sincerely,


Aaron W. Young
Laboratory Manager

Project #: 6-917-180960

BACT = Bacteriological
CONV = Conventionals

MET = Metals
ORG = Organics

NUT=Nutrients
DEM=Demand

MIN=Minerals

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ANALYSIS REPORT

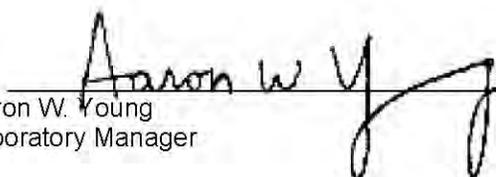
AMEC FOSTER WHEELER
11810 NORTH CREEK PKWY N
BOTHELL, WA 98011
Attention: KONRAD MUELLER/TODD WENTWORTH
Project Name: CENTRAL KITSAP HS/MS
Project #: 6-917-180960
All results reported on an as received basis.

Date Received: 02/22/17
Date Reported: 3/1/17

AMTEST Identification Number 17-A002444
Client Identification IT-4/G-4E-9
Sampling Date 02/20/17

Conventionals

PARAMETER	RESULT	UNITS	Q	D.L.	METHOD	ANALYST	DATE
Cation Exchange Capacity	1.2	meq/100g		0.5	SW-846 9081	AY	03/01/17
Organic Matter	0.7	%		0.1	ASTM D 2974	SW	02/24/17


Aaron W. Young
Laboratory Manager

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 13600 NE 126th PL
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 Kirkland, WA, 98034
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QC Summary for sample number: 17-A002444

DUPLICATES

SAMPLE #	ANALYTE	UNITS	SAMPLE VALUE	DUP VALUE	RPD
17-A002492	Cation Exchange Capacity	meq/100g	3.1	2.7	14.
17-A002444	Organic Matter	%	0.7	0.6	15.

STANDARD REFERENCE MATERIALS

ANALYTE	UNITS	TRUE VALUE	MEASURED VALUE	RECOVERY
Cation Exchange Capacity	meq/100g	4.0	3.9	97.5 %

BLANKS

ANALYTE	UNITS	RESULT
Cation Exchange Capacity	meq/100g	< 0.5
Organic Matter	%	< 0.1



AmTest Chain of Custody Record

13600 NE 126th PL, Suite C, Kirkland, WA 98034

Ph (425) 885-1664 Fx (425) 820-0245

www.amtestlab.com

Chain of Custody No. **30336**

Client Name & Address: <i>Amec Foster Wheeler</i>	Invoice To:
Contact Person: <i>Kenneth Mueller / Todd Wentworth</i>	Invoice Contact:
Phone No: <i>425-368-1000</i>	PO Number:
Fax No:	Invoice Ph/Fax:
E-mail: <i>Todd.wentworth@amec.fw.com</i> <i>Kenneth.Mueller@amec.fw.com</i>	Invoice E-mail:
Report Delivery: (Choose all that apply) Mail / Fax / <u>Email</u> / Posted Online	Data posted to online account: YES / NO Web Login ID:

Special Instructions:

Requested TAT: (Rush must be pre-approved by lab)
Standard RUSH (5 Day / 3 Day / 48 HR / 24 HR) Temperature upon Receipt: *18*

Project Name: <i>Central Kitsap HS/MS</i>		Date Sampled	Time Sampled	Matrix	No. of containers	Analysis Requested										QA/QC
Project Number: <i>6-917-160960</i>						<i>LEC</i>	<i>organic content</i>									
AmTest ID	Client ID (35 characters max)															
2444																
<i>2444</i>	<i>IT-4/64e-9-</i>	<i>2/20/17</i>		<i>soil</i>	<i>1</i>	<i>X</i>	<i>X</i>									

Collected/Relinquished By: <i>Kel Mueller</i>	Date <i>2/22/17</i>	Time <i>4:45</i>	Received By: <i>PK</i>	Date <i>2/22/17</i>	Time <i>1645</i>
Relinquished By:	Date	Time	Received By:	Date	Time
Relinquished By:	Date	Time	Received By:	Date	Time

COMMENTS:

current



APPENDIX C

PIT Procedures and Data Sheets

APPENDIX C
PIT PROCEDURES AND DATA SHEETS
Central Kitsap High School and Middle School Redevelopment
Silverdale, Washington
Project No. 6-917-18096-0

At each infiltration testing location a test pit was excavated to the target infiltration depth. A soil sample of the subgrade soils at the test depth was taken to determine index properties of the proposed infiltration soils. At the test depth for each test location, a 6-foot-diameter, steel-ring infiltrometer with cover was tamped approximately 6 inches into the undisturbed soil at the bottom of the test pit by the bucket of the excavator. The area around the outside of the ring was then backfilled with soil up to the top of the ring. Water was pumped into the steel ring infiltrometer through a 2-inch- or $\frac{3}{4}$ -inch-diameter hose, discharging into the bottom of the test ring through a machine-slotted polyvinyl chloride screen to minimize scour with the flow rate monitored by a rotameter, accurate to 0.1 gallon per minute. Water depths within the infiltrometer were measured by a vertical measuring rod, marked in inches or 0.1-foot increments.

Once the equipment had been set up, water was pumped into the ring, noting the flow rate and time. A maximum of 6 to 13 inches of water was maintained within the ring during the “pre-soak” period. For IT-1 through IT-4, the pre-soak time was 6 hours. Infiltration testing began once stabilized readings had been obtained for a steady state at the appropriate flow rate for a period of 1 hour. The flow rate, total volume of water added, and the time were regularly recorded during the pre-soaking and steady-state periods. Infiltration testing began immediately after completing the pre-soaking and steady-state phase. At that time, the flow of water into the steel ring was halted the drop in water level was monitored every 15 minutes (falling head test) until the 6-foot-diameter ring was empty or nearly empty of water. The infiltration data collected for each completed PIT are presented in this appendix.

Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-1 (South Parking Lot)
Date: 12/21/16

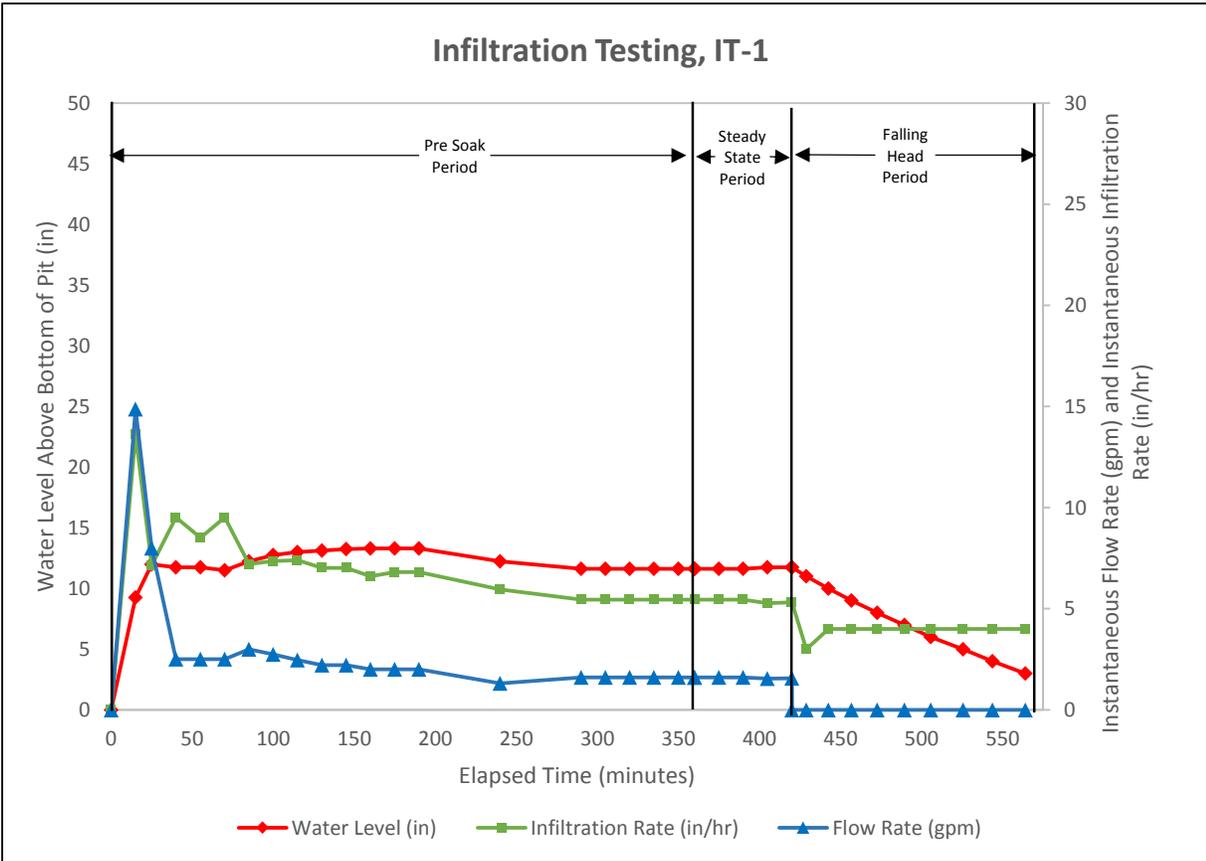
Test Pit Dimensions			
	Length (ft)	Width (ft)	Radius (ft)
Base			3
Surface			N/A
Area of Pit Base (in²)	4071.504079		

Infiltration Information	
Falling Head Minimum (in/hr)	3.0
Falling Head Maximum (in/hr)	5.0
Falling Head Average (in/hr)	3.8
Constant Head Infiltration (in/hr)	5.4

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)
Pre Soak Period (6 hrs)	9:40 AM	0	0	0	0	-	-
	9:55 AM	15	9.25	14.87	37661.4127	223.05	13.6
	10:05 AM	25	12	8	48858.0489	80.00	10.7
	10:20 AM	40	11.75	2.5	47840.1729	37.50	9.5
	10:35 AM	55	11.75	2.5	47840.1729	37.50	8.5
	10:50 AM	70	11.5	2.5	46822.2969	37.50	9.5
	11:05 AM	85	12.25	3	49875.925	45.00	7.2
	11:20 AM	100	12.75	2.75	51911.677	41.25	7.4
	11:35 AM	115	13	2.47	52929.553	37.05	7.4
	11:50 AM	130	13.125	2.21	53438.491	33.15	7.0
	12:05 PM	145	13.25	2.21	53947.429	33.15	7.0
	12:20 PM	160	13.3	2	54151.0043	30.00	6.6
	12:35 PM	175	13.3	2	54151.0043	30.00	6.8
	12:50 PM	190	13.3	2	54151.0043	30.00	6.8
	1:05 PM	205	13.5	1	54965.3051	15.00	2.6
	1:25 PM	225	12.625	1.25	51402.739	25.00	6.9
	1:40 PM	240	12.25	1.31	49875.925	19.65	6.0
	1:55 PM	255	12.875	1.3	52420.615	19.50	1.9
	2:15 PM	275	11.625	1.6	47331.2349	32.00	9.2
	2:30 PM	290	11.625	1.6	47331.2349	24.00	5.4
2:45 PM	305	11.625	1.6	47331.2349	24.00	5.4	
3:00 PM	320	11.625	1.6	47331.2349	24.00	5.4	
3:15 PM	335	11.625	1.6	47331.2349	24.00	5.4	
3:30 PM	350	11.625	1.6	47331.2349	24.00	5.4	
3:40 PM	360	11.625	1.6	47331.2349	16.00	5.4	
Steady State Period (1 hr)	3:40 PM	360	11.625	1.6	47331.2349	16.0	5.4
	3:55 PM	375	11.625	1.6	47331.2349	24.000	5.4
	4:10 PM	390	11.625	1.6	47331.2349	24.000	5.4
	4:25 PM	405	11.75	1.55	47840.1729	23.250	5.3
	4:40 PM	420	11.75	1.56	47840.1729	23.400	5.3

Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-1 (South Parking Lot)
Date: 12/21/16

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)
Falling Head Period (2 hr, 21 min)	4:49 PM	429	11	0	44786.5449	0.0	5.0
	5:02 PM	443	10	0	40715.0408	0.0	4.4
	5:17 PM	457	9	0	36643.5367	0.0	4.2
	5:39 PM	473	8	0	32572.0326	0.0	3.8
	5:56 PM	490	7	0	28500.5286	0.0	3.6
	6:12 PM	506	6	0	24429.0245	0.0	3.7
	6:32 PM	526	5	0	20357.5204	0.0	3.0
	6:50 PM	544	4	0	16286.0163	0.0	3.3
	7:10 PM	564	3	0	12214.5122	0.0	3.0



Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-2 (Alternative HS Parking Lot - East End)
Date: 12/22/16

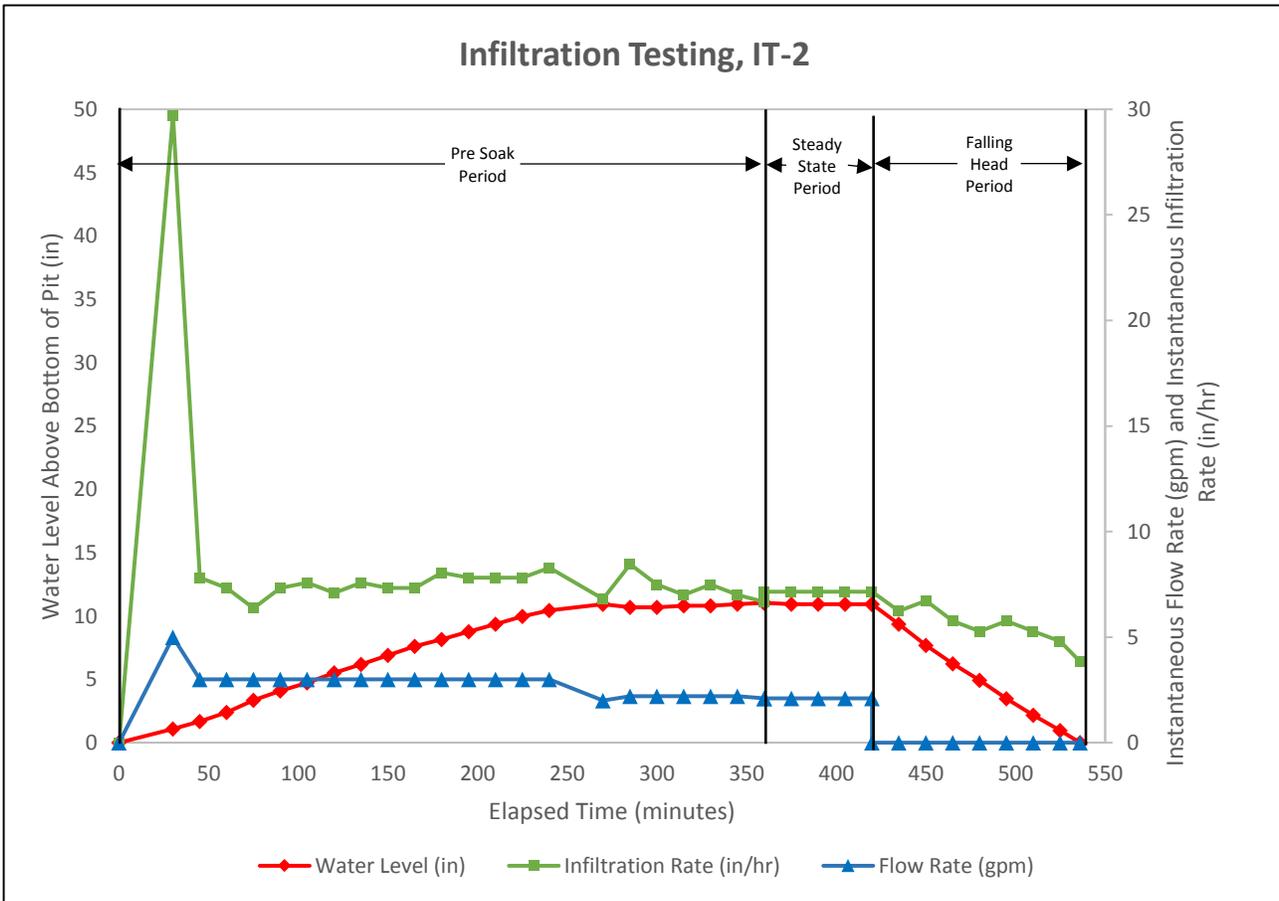
Test Pit Dimensions			
	Length (ft)	Width (ft)	Radius (ft)
Base			3
Surface			N/A
Area of Pit Base (in²)		4071.504079	

Infiltration Information	
Falling Head Minimum (in/hr)	4.8
Falling Head Maximum (in/hr)	6.7
Falling Head Average (in/hr)	5.6
Constant Head Infiltration (in/hr)	7.1

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)	
Pre Soak Period (6 hrs)	8:45 AM	0	0	0	0	-	-	
	9:15 AM	30	1.08	5	4397.22441	150.00	14.9	
	9:30 AM	45	1.68	3	6840.12685	45.00	7.8	
	9:45 AM	60	2.4	3	9771.60979	45.00	7.3	
	10:00 AM	75	3.36	3	13680.2537	45.00	6.4	
	10:15 AM	90	4.08	3	16611.7366	45.00	7.3	
	10:30 AM	105	4.74	3	19298.9293	45.00	7.6	
	10:45 AM	120	5.52	3	22474.7025	45.00	7.1	
	11:00 AM	135	6.18	3	25161.8952	45.00	7.6	
	11:15 AM	150	6.9	3	28093.3781	45.00	7.3	
	11:30 AM	165	7.62	3	31024.8611	45.00	7.3	
	11:45 AM	180	8.16	3	33223.4733	45.00	8.1	
	12:00 PM	195	8.76	3	35666.3757	45.00	7.8	
	12:15 PM	210	9.36	3	38109.2782	45.00	7.8	
	12:30 PM	225	9.96	3	40552.1806	45.00	7.8	
	12:45 PM	240	10.44	3	42506.5026	45.00	8.3	
	Possible anomaly in flow of stage reading	1:00 PM	255	10.92	2	44460.8245	30.00	4.9
		1:15 PM	270	10.92	2	44460.8245	30.00	6.8
		1:30 PM	285	10.68	2.2	43483.6636	33.00	8.4
		1:45 PM	300	10.68	2.2	43483.6636	33.00	7.5
	2:00 PM	315	10.8	2.2	43972.2441	33.00	7.0	
	2:15 PM	330	10.8	2.2	43972.2441	33.00	7.5	
	2:30 PM	345	10.92	2.2	44460.8245	33.00	7.0	
	2:45 PM	360	11.04	2.1	44949.405	31.50	6.7	
Steady State Period (1 hr)	2:45 PM	360	11.04	2.1	44949.405	31.50	7.1	
	3:00 PM	375	10.92	2.1	44460.8245	31.500	7.1	
	3:15 PM	390	10.92	2.1	44460.8245	31.500	7.1	
	3:30 PM	405	10.92	2.1	44460.8245	31.500	7.1	
	3:45 PM	420	10.92	2.1	44460.8245	31.500	7.1	

Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-2 (Alternative HS Parking Lot - East End)
Date: 12/22/16

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)
Falling Head Period (1 hr, 41 min)	4:00 PM	435	9.36	0	38109.2782	0.0	6.2
	4:15 PM	450	7.68	0	31269.1513	0.0	6.7
	4:30 PM	465	6.24	0	25406.1855	0.0	5.8
	4:45 PM	480	4.92	0	20031.8001	0.0	5.3
	5:00 PM	495	3.48	0	14168.8342	0.0	5.8
	5:15 PM	510	2.16	0	8794.44881	0.0	5.3
	5:30 PM	525	0.96	0	3908.64392	0.0	4.8
	5:41 PM	536	0	0	0	0.0	5.2



Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-3 (Alternative High School - SW Corner of Building)
Date: 12/23/16

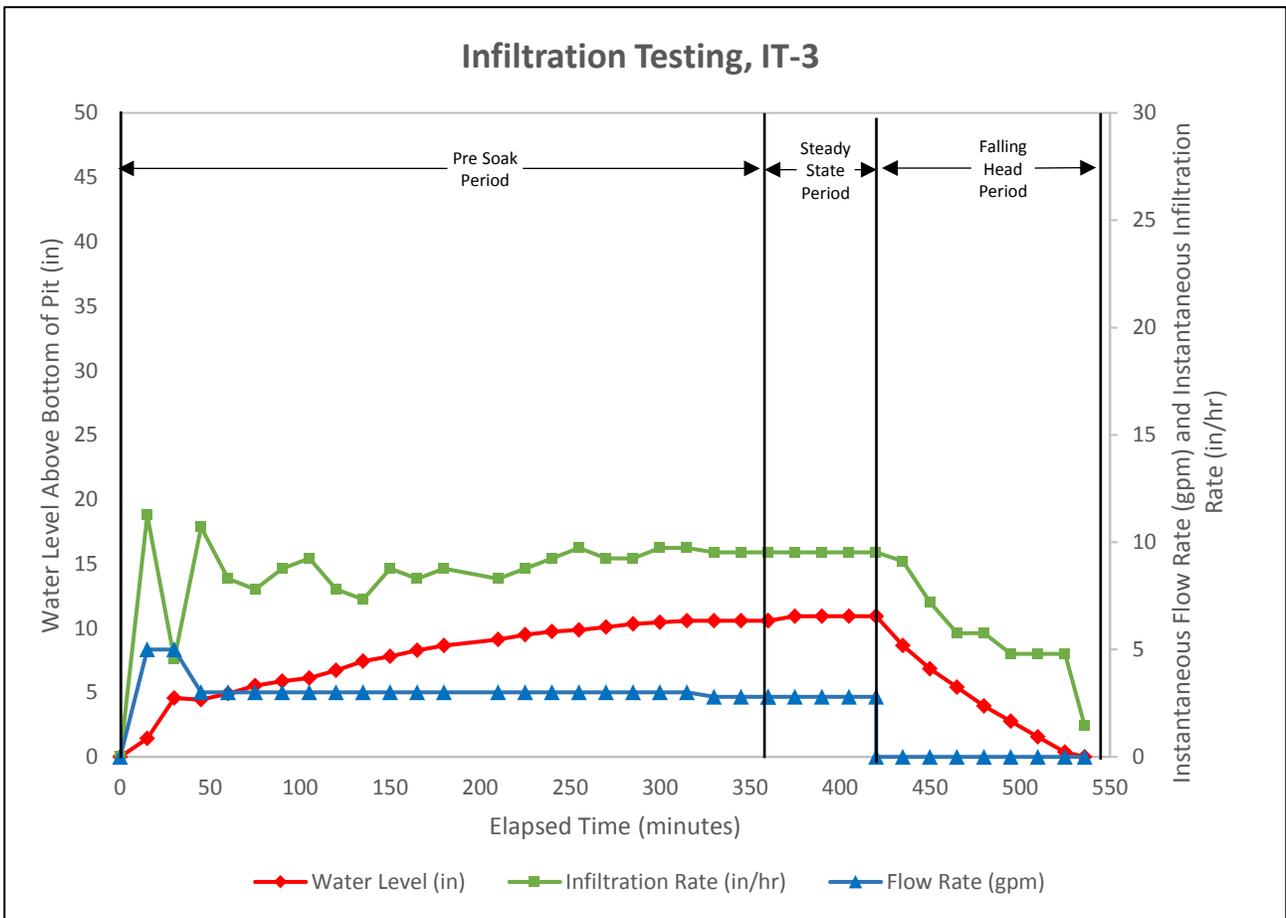
Test Pit Dimensions			
	Length (ft)	Width (ft)	Radius (ft)
Base			3
Surface			N/A
Area of Pit Base (in ²)		4071.504079	

Infiltration Information	
Falling Head Minimum (in/hr)	2.0
Falling Head Maximum (in/hr)	9.1
Falling Head Average (in/hr)	5.5
Constant Head Infiltration (in/hr)	9.2

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)
	7:25 AM	0	0	0	0	-	-
	7:40 AM	15	1.44	5	5862.96587	75.00	11.3
Possible anomaly in flow of stage reading	7:55 AM	30	4.56	5	18566.0586	75.00	4.5
	8:10 AM	45	4.44	3	18077.4781	45.00	10.7
	8:25 AM	60	4.92	3	20031.8001	45.00	8.3
	8:40 AM	75	5.52	3	22474.7025	45.00	7.8
	8:55 AM	90	5.88	3	23940.444	45.00	8.8
	9:10 AM	105	6.12	3	24917.605	45.00	9.3
	9:25 AM	120	6.72	3	27360.5074	45.00	7.8
	9:40 AM	135	7.44	3	30291.9903	45.00	7.3
	9:55 AM	150	7.8	3	31757.7318	45.00	8.8
Pre Soak Period (6 hrs)	10:10 AM	165	8.28	3	33712.0538	45.00	8.3
	10:25 AM	180	8.64	3	35177.7952	45.00	8.8
Possible anomaly in flow of stage reading	10:40 AM	195	8.64	3	35177.7952	45.00	10.2
	10:55 AM	210	9.12	3	37132.1172	45.00	8.3
	11:10 AM	225	9.48	3	38597.8587	45.00	8.8
	11:25 AM	240	9.72	3	39575.0196	45.00	9.3
	11:40 AM	255	9.84	3	40063.6001	45.00	9.7
	11:55 AM	270	10.08	3	41040.7611	45.00	9.3
	12:10 PM	285	10.32	3	42017.9221	45.00	9.3
	12:25 PM	300	10.44	3	42506.5026	45.00	9.7
	12:40 PM	315	10.56	3	42995.0831	45.00	9.7
	12:55 PM	330	10.56	2.8	42995.0831	42.00	9.5
	1:10 PM	345	10.56	2.8	42995.0831	42.00	9.5
	1:25 PM	360	10.56	2.8	42995.0831	42.00	9.5
Steady State Period (1 hr)	1:25 PM	360	10.56	2.8	42995.0831	42.00	9.5
	1:40 PM	375	10.92	2.8	44460.8245	42.000	8.1
	1:55 PM	390	10.92	2.8	44460.8245	42.000	9.5
	2:10 PM	405	10.92	2.8	44460.8245	42.000	9.5
	2:25 PM	420	10.92	2.8	44460.8245	42.000	9.5

Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-3 (Alternative High School - SW Corner of Building)
Date: 12/23/16

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)
Falling Head Period (1 hr, 45 min)	2:40 PM	435	8.64	0	35177.7952	0.0	9.1
	2:55 PM	450	6.84	0	27849.0879	0.0	7.2
	3:10 PM	465	5.4	0	21986.122	0.0	5.8
	3:25 PM	480	3.96	0	16123.1562	0.0	5.8
	3:40 PM	495	2.76	0	11237.3513	0.0	4.8
	3:55 PM	510	1.56	0	6351.54636	0.0	4.8
	4:10 PM	525	0.36	0	1465.74147	0.0	4.8
	4:25 PM	536	0	0	0	0.0	2.0



Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-4 (Science Building - North Gravel Parking Area)
Date: 2/20/17

Test Pit Dimensions			
	Length (ft)	Width (ft)	Radius (ft)
Base			3
Surface			N/A
Area of Pit Base (in ²)		4071.504079	

Infiltration Information	
Falling Head Minimum (in/hr)	0.8
Falling Head Maximum (in/hr)	1.5
Falling Head Average (in/hr)	1.0
Constant Head Infiltration (in/hr)	1.3

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)
	9:10 AM	0	0	0	0	-	-
	9:25 AM	15	1.25	3.3	5089.3801	49.50	6.2
	9:40 AM	30	3.25	3.3	13232.3883	49.50	3.2
	9:55 AM	45	5.13	3.3	20886.8159	49.50	3.7
Changed out flow meter (turned off water)	10:10 AM	60	4.62	0	18810.3488	0.00	2.0
	10:25 AM	75	5.75	2	23411.1485	30.00	2.3
	10:40 AM	90	6.88	1.75	28011.9481	26.25	1.4
	10:55 AM	105	7.75	1.5	31554.1566	22.50	1.6
Pre Soak Period (6 hrs)	11:10 AM	120	8.31	1.25	33834.1989	18.75	2.0
	11:25 AM	135	8.75	1	35625.6607	15.00	1.6
	11:40 AM	150	9.06	0.75	36887.827	11.25	1.3
	11:55 AM	165	9.38	0.75	38190.7083	11.25	1.3
	12:10 PM	180	9.56	0.75	38923.579	11.25	1.8
	12:25 PM	195	9.75	0.65	39697.1648	9.75	1.5
	12:40 PM	210	10	0.65	40715.0408	9.75	1.2
	12:55 PM	225	9.88	0.3	40226.4603	4.50	1.5
	1:10 PM	240	9.88	0.43	40226.4603	6.45	1.5
	1:25 PM	255	10	0.45	40715.0408	6.75	1.1
	1:40 PM	270	10	0.43	40715.0408	6.45	1.5
	1:55 PM	285	10.13	0.44	41244.3363	6.60	1.0
	2:10 PM	300	10.13	0.44	41244.3363	6.60	1.5
	2:25 PM	315	10.19	0.44	41488.6266	6.60	1.3
	2:40 PM	330	10.25	0.45	41732.9168	6.75	1.3
		1:55 PM	345	10.25	0.44	41732.9168	6.60
	3:10 PM	360	10.31	0.45	41977.2071	6.75	1.3
Steady State Period (1 hr)	3:10 PM	360	10.31	0.45	41977.2071	6.75	1.3
	3:25 PM	375	10.31	0.45	41977.2071	6.75	1.5
	3:40 PM	390	10.38	0.45	42262.2123	6.75	1.3
	3:55 PM	405	10.48	0.45	42669.3627	6.75	1.1
	4:10 PM	420	10.48	0.45	42669.3627	6.75	1.5

Job Name: Central Kitsap High School / Middle School
Job Number: 6-917-180960
Infiltration Test: IT-4 (Science Building - North Gravel Parking Area)
Date: 2/20/17

	Time (hh:mm)	Elapsed Time (min)	Water Level Stage (in)	Flow Rate (gal/min)	Pit Volume (in ³)	Volume Added (Gal)	Infiltration Rate (in/hr)
Falling Head Period (3 hr)	4:10 PM	420	10.48	0	42669.3627	0.0	1.5
	4:32 PM	442	10	0	40715.0408	0.0	1.3
	5:30 PM	500	9	0	36643.5367	0.0	1.0
	5:48 PM	518	8.75	0	35625.6607	0.0	0.8
	6:04 PM	534	8.5	0	34607.7847	0.0	0.9
	6:20 PM	550	8.25	0	33589.9087	0.0	0.9
	6:36 PM	566	8	0	32572.0326	0.0	0.9
	6:53 PM	583	7.75	0	31554.1566	0.0	0.9
	7:10 PM	600	7.5	0	30536.2806	0.0	0.9

